# Contra Costa I-680 (North) Northbound Design Alternative Assessment 

Contra Costa County, California
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## 1. EXECUTIVE SUMMARY

The Metropolitan Transportation Commission (MTC), in cooperation with the Contra Costa Transportation Authority (CCTA) initiated the I-680 (North) Northbound (NB) Design Alternative Assessment (DAA) to identify and evaluate a range of alternatives between Livorna Road and State Route (SR) 242 that would address the existing l-680 northbound managed lane gap, improve traffic operations and relieve congestion on northbound I-680 in Contra Costa County.

The scope of the DAA includes the identification and development of geometric concepts, operational analysis and feasibility assessment of selected alternatives. A Technical Advisory Committee (TAC) consisting of MTC, CCTA, and local officials was formed in October 2015 to provide input and guide the DAA Team on the identification and feasibility of geometric concepts and traffic analysis alternatives. The goal of the DAA is to identify a set of feasible alternatives that can be included for further study in the next project delivery phase of a future northbound I680 project.

## Geometric Improvement Concepts

Working with the TAC, a wide range of solutions were considered to address the I-680 northbound gap, including several operational improvement strategies that would complement the managed lane operation and provide additional congestion relief benefits.

The TAC eliminated design concepts based on the following criteria:

- Substantial design constraints/issues related to meeting the standards presented in the California Highway Design Manual
- Significant right-of-way (ROW) constraints
- TAC consensus, based on projected volume forecasts, that the concept would not meet the primary project objectives to improve managed lane connectivity or traffic operations

Once the concepts were evaluated based on the criteria, the TAC determined how to arrange the remaining concepts into alternatives for traffic analysis.

## Traffic Analysis Alternatives

A total of ten alternatives (including No Build) were studied as part of this evaluation under year 2020 conditions. Due to design constraints along the corridor only two of the ten alternatives (Alternatives 8 and 9 ) would provide a continuous managed lane (eliminate the gap). Six of the alternatives would reduce the length of the gap (Alternatives 3, 4, 5, 6, 6A, and 7). Two alternatives (Alternative 1 and 2) would maintain the existing gap. Table E-1 presents an overview of the key features and assumptions for each alternative. A schematic of each of the evaluated alternatives is presented in Appendix A.

Table E-1: Traffic Analysis Alternatives

| Alternative | Title | Key Features And Assumptions |
| :---: | :---: | :---: |
| 1 | No Build | This is the baseline alternative. Assumes that by year 2020 the northbound I-680 Express Lane is operational on the southern end of the study corridor. The managed lanes gap is about 7.5 miles. |
| 2 | Adaptive Ramp Metering | This alternative assumes adaptive ramp metering. This is also included as part of Alternatives 3 through 9. |
| 3 | Express Lane: GP <br> Lane Conversion with <br>  <br> Ride Investments | This alternative converts a General Purpose (GP) Lane to an Express Lane. This alternative assumes a 20\% mode shift from SOV to HOV and transit, as a result of an expanded Express Lane system and future corridor investments in transit and park-n-ride facilities with shuttle service to BART stations. This alternative also assumes a policy change for the HOV Lane from 2+ persons to $3+$ persons and an increase in HOV occupancy from an average of about 2.2 persons under existing conditions to 4.0 persons. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 4 | Express Lane: GP Lane Conversion Plus C-D System with Transit and Park \& Ride Investments | This alternative is similar to Alternative 3 but also provides a Collector-Distributor (C-D) road system to service the North Main Street off-ramp, North Main Street on-ramp, and Treat Boulevard off-ramp to eliminate mainline weaving and capacity issues at this location. This alternative also assumes the $20 \%$ mode shift from SOV to HOV and transit, as well as the HOV occupancy change included in Alternative 3. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 5 | Express Lane <br> Extension and GP <br> Lane Widening | This alternative provides outside widening on both the north side and south side of the SR 24 interchange to increase the length of the Express Lane and reduce the existing gap. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |


| 6 | Express Lane <br> Extension and GP <br> Lane Widening Plus C-D System | This alternative is similar to Alternative 5 except it also provides a C-D road system to eliminate the mainline weaving between the North Main Street on-ramp and Treat Boulevard off-ramp. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| :---: | :---: | :---: |
| 6A | Express Lane: GP <br> Lane Conversion and GP Lane Widening Plus C-D System | This scenario is similar to Alternative 6 except it does not include a managed lane extension from N . Main St to SR 242 and only includes an express lane conversion from Livorna Rd to SR 24. The managed lanes gap would be shorten from 7.5 miles to about 4.5 miles. This alternative would be an initial phase of Alternative 6. |
| 7 | Express Lane <br> Extension and GP <br> Lane Widening Plus <br> C-D System and <br> Ygnacio I/C <br> Reconfiguration | This alternative is similar to Alternative 6 except it does not include the mainline widening between Olympic Boulevard and Ygnacio Boulevard and instead reconfigures the Ygnacio Boulevard off-ramp. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 8 | Contra-Flow Plus <br> Express Lane <br> Extension and GP <br> Lane Widening | This alternative provides a contra flow lane (by using the southbound express lane during the PM peak) to provide a continuous Express Lane with no gap. |
| 9 | SR 24 and Ygnacio I/C Reconfiguration Plus Express Lane Extension and GP Lane Widening | This alternative would reconfigure the I-680/SR 24 interchange (SR 24 would join I-680 on the right hand side as opposed to the left-side) to provide a continuous Express Lane with no gap. |

## Traffic Operational Analysis Findings

Exhibit E-1 presents the average peak period travel time by mode (HOV, Toll, and SOV) and alternative. Northbound I-680 travel time is measured for drivers traveling between I-580 and State Route 4 (SR 4), a distance of approximately 23 miles. Under Alternative 1 the HOV and tolled vehicles average peak period travel time would be about 41 minutes while the SOV
average peak period travel time would be about 80 minutes in year 2020. The free-flow travel time (average speed of 65 mph ) is about 21 minutes. All of the alternatives are anticipated to reduce average peak period travel times for all modes of travel compared to Alternative 1.

The greatest reduction to HOV/TOLL travel times would be provided under Alternatives 4, 6, 7, 8 , and 9 where the average peak period travel time would be reduced to 22 minutes. The largest reduction to SOV travel times would be provided under Alternative 4. Alternative 4 would improve traffic operations via the C-D road system at one of the major existing bottlenecks on northbound I-680 (North Main Street on-ramp to Treat Boulevard off-ramp). Alternative 4 also assumes an overall $20 \%$ reduction in vehicle demand compared to Alternative 1 as a result of mode shift (SOV to HOV and transit) and future corridor investments in transit and park-n-ride facilities that will increase the overall average HOV occupancy on the corridor (from about 2.2 under Alternative 1 conditions to 4.0). Alternative 3 also assumed a similar 20\% reduction in vehicle demand compared to Alternative 1. As a result, travel times under Alternative 3 for all modes would be reduced compared to Alternative 1. Alternatives 6, 6A, 7, 8, and 9 would also provide a substantial reduction to travel times. The reduction in travel times under these alternatives is primarily attributed to operational and capacity improvements along corridor. Alternative 2 , ramp metering alone, would provide the least reduction to travel times.

EXHIBIT E-1: Average Peak Period Travel Times by Mode of Travel (3:00 PM to 7:00 PM)


## Conclusion and Recommendations

This DAA memo discusses the process the TAC has taken to determine and evaluate geometric improvement concepts and develop alternatives for traffic analysis. A summary of the assessment used to determine the recommended alternatives for the next phase of study is provided below.

Table E-2 shows the studied alternatives and the associated benefit-cost ratio based on estimated average construction costs and an operational benefit factor based on the traffic analysis results.

## Table E-2: Alternative Benefit - Cost Ratios

| ALTERNATIVE | AVERAGE COST* <br> (MILLIONS \$) | OPERATIONAL BENEFIT FACTOR | $\begin{aligned} & \text { BENEFIT- } \\ & \text { COST RATIO } \end{aligned}$ | BENEFITCOST RANK |
| :---: | :---: | :---: | :---: | :---: |
| Alternative 2 | 17.5 | 240 | 13.7 | 1 |
| Alternative 3 | 90 | 790 | 8.8 | 2 |
| Alternative 4 | 144 | 999 | 6.9 | 3 |
| Alternative 6A | 112.5 | 760 | 6.8 | 4 |
| Alternative 8 | 240 | 970 | 4.0 | 5 |
| Alternative 5 | 230 | 870 | 3.8 | 6 |
| Alternative 6 | 275 | 970 | 3.5 | 7 |
| Alternative 7 | 375 | 970 | 2.6 | 8 |
| Alternative 9 | 800 | 970 | 1.2 | 9 |

* Costs shown are construction estimates only and do not include support costs

The benefit/cost analysis is summarized as follows:

- Alternative 2 has the highest benefit-cost ratio as a result of a modest operational benefit but with a substantially lower cost compared to the other alternatives. Some key conclusions are :
o This alternative is included in all the other studied alternatives, except Alternative 1.
o Adaptive ramp metering if desired can be separated and delivered as an initial delivery project for NB I-680.
- Alternatives 4, 6 and 8 all provide similar substantial operational benefits for NB I-680 under a different set of key assumptions and/or geometric improvement concepts:
o Alternative 4 assumes a $20 \%$ mode shift (SOV to HOV and transit, through investments in transit and park \& ride services) with the GP Lane Conversion

Design Concept C (North Main to SR 242). It has a lower cost compared to Alternatives 6 and 8.
o Alternatives 4 and 6 both include a C-D Road (North Main to Treat).
o Alternative 6 and 8 include the Managed Lane Extension (North Main to SR 242).
o Alternative 8 includes a Contra Flow Lane and has a lower cost than Alternative 9 (the other alternative that closely the gap completely).
o Based on these findings and comparison to the other remaining alternatives these three alternatives are ideal for further study.

- Alternative 3 can be considered a subset (or an initial phase) of Alternative 4 as it also assumes a $20 \%$ mode shift and includes the same geometric improvement concepts except the C-D Road (North Main to Treat). Some key conclusions for Alternative 3 compared to Alternative 4
o Alternative 3 has a higher benefit-cost ratio than Alternative 4.
o Although Alternative 3 has a higher benefit-cost ratio, Alternative 4 is better suited for further study as an analysis can be performed with and without the C-D Road and effectively evaluate Alternative 3 and 4.
- Alternatives 5 and 6A can be considered subsets (or an initial phases) of Alternative 6 due to the following:
o Alternative 5 includes all of the geometric improvement concepts as Alternative 6 except the C-D Road (North Main to Treat).
o Alternative 6A includes all of the geometric improvement concepts as Alternative 6 except the Managed Lane Extension (North Main to SR 242).
0 Alternative 6 is better suited for further study vs. Alternatives 5 and 6A as Alternative 6 can be evaluated with and without the C-D road and Managed Lane Extension and effectively evaluate Alternatives 5, 6A, and 6.
- Alternatives 8 and 9 provide improvements that close the I-680 NB managed lane gap:
o Alternative 8 is better suited for further study compared to Alternative 9 as it fully meets the project objective (closing the gap in the managed lane) at a lower cost.
o Alternative 9 has the lowest benefit-cost ratio, highest cost, and substantial risks and challenges that make this alternative not suitable for further study.
- Alternatives 7 and 9 have the lowest benefit-cost ratios along with several risks that include:
o Significant project delivery risks and construction challenges.
o Alternatives 7 and 9 are not ideal alternatives to be carried through for further study due to the costs, risks and approval challenges.


## RECOMMENDED ALTERNATIVES

This DAA recommends three proposed alternatives be studied and compared to the No Build alternative to identify the preferred alternative during the next project delivery phase:

- No Build
- Alternative 4 - GP Lane Conversion Plus C-D System with Transit and Park \& Ride Investments
- Alternative 6 - Express Lane Extension and GP Lane Widening Plus C-D System
- Alternative 8 - Contra-Flow Plus Express Lane Extension and GP Lane Widening


## 2. INTRODUCTION

The Metropolitan Transportation Commission (MTC), in cooperation with the Contra Costa Transportation Authority (CCTA) initiated the I-680 (North) Northbound (NB) Design Alternative Assessment (DAA) to identify and evaluate a range of alternatives between Livorna Road and State Route (SR) 242 that would address the existing I-680 NB managed lane gap, improve traffic operations and relieve congestion on northbound I-680 in Contra Costa County.

The scope of the DAA includes the identification and development of geometric concepts, operational analysis and feasibility assessment of selected alternatives. A Technical Advisory Committee (TAC) consisting of MTC, CCTA, and local officials was formed in October 2015 to provide input and guide the DAA Team on the identification and feasibility of geometric concepts and traffic analysis alternatives. The goal of the DAA is to identify a set of feasible alternatives that can be included for further study in the next project delivery phase of a future northbound I680 project.

This technical memorandum, the Design Alternative Assessment Memo, summarizes the assessment of the Geometric Improvement Concepts and Traffic Analysis Alternatives developed during this study and provides a recommendation of project alternatives to be carried to the next phase for project delivery.

## 3. BACKGROUND

## I-680 Corridor

I-680 is a six- to ten-lane major north-south freeway connecting the Bay Area with I-80 and I580 for travel to and from the Central Valley and Sacramento metropolitan area. Within the Bay Area, I-680 passes through Santa Clara, Alameda, Contra Costa, and Solano counties. I-680 is functionally classified as an Urban Principle Arterial-Interstate Freeway and is considered a Lifeline Highway route along the entire length of the interstate freeway. The study segment of I680 is heavily traveled as it is utilized by commuters, recreational travelers, and public transit services.

## I-680 Managed Lanes System

Managed lanes include both High Occupancy Vehicle (HOV) lanes and express lanes. HOV lanes are provided on several sections of I-680 within Contra Costa County from the BeniciaMartinez Bridge in the north to Alcosta Boulevard in the south. These lanes are open to only HOV (carpool) eligible vehicles on weekdays between the hours of 5:00 AM to 9:00 AM and 3:00 PM to 7:00 PM. Express lanes, also known as High Occupancy Toll (HOT) lanes, are either planned or currently under construction on I-680 in Contra Costa County. Express lanes
are HOV lanes that are free to carpools, buses, motorcycles, and other eligible vehicles, and available to single occupant vehicles (SOV) that choose to pay a toll.

There will be a continuous express lane in Contra Costa County on I-680 in the southbound direction from the Marina Vista Avenue interchange to the Alcosta Boulevard interchange (approximately 30 miles) within the next five years. This express lane will provide increased operational flexibility to respond to southbound traffic demand growth and enhance southbound mobility. The southbound express lane is part of the Metropolitan Transportation Commission's vision to eventually provide 550 miles of express lane network in the Bay Area.

In the northbound direction there is an HOV lane in the southern part of the county between Alcosta Boulevard and Livorna Road (southern managed lane) which is currently being converted to an express lane, and an HOV lane in the northern part of the county from SR242 to about 1-mile south of Benicia-Martinez bridge toll plaza (northern managed lane). There is a 7.5 mile gap between these managed lane segments which diminishes the effectiveness of the northbound managed lane. The purpose of this study is to identify and evaluate improvements to $\mathrm{I}-680$ in this gap.

The northbound gap in the Contra Costa County I-680 Managed Lanes is shown in Figure 3-1.

Figure 3-1: Contra Costa County I-680 Managed Lanes


## Problems, Deficiencies, Constraints:

As identified in MTC's 2014 Bay Area Congested Segments, traffic during the northbound I-680 PM peak period between Crow Canyon Road and Treat Boulevard has been ranked as the fifth worst commute corridor in the San Francisco Bay Area. There are two primary bottlenecks between the Livorna Road on-ramp and Rudgear Road off-ramp and between the North Main Street (Lawrence Way) on-ramp and Treat Boulevard off-ramp. These bottlenecks result in congestion between 3:30 PM and 7 PM and vehicle queues in excess of 12 miles.

At the heart of the northbound congestion near the City of Walnut Creek, a three-lane freeway ramp connector from SR 24 eastbound connects with I-680 northbound from the left-hand side in the median. Immediately north of the ramp connector, elevated tracks for the Bay Area Rapid Transit (BART) cross I-680 resulting in several structural columns in the median and the outside shoulder of northbound I-680. These two elements create significant design constraints that make it challenging to widen the mainline to create a continuous northbound express lane through the existing managed lane gap.

## 4. GEOMETRIC CONCEPTS DEVELOPMENT

In collaboration with the TAC, a wide range of solutions were considered to close the northbound gap, including several operational improvement strategies that would complement the managed lanes and provide additional congestion relief benefits. Concepts and findings were presented by the Project Team and explored with the TAC over a series of five meetings held between October 2015 and March 2016. The Project Team and TAC members are shown in Table 4-1.

Table 4-1: Project Team and Technical Advisory Committee Members

| Name Title | Technical Advisory Committee |  |
| :--- | :--- | :--- |

Table 4-2 and 4-3 presents the geometric improvement concepts that were investigated with the TAC and identifies those that were eliminated prior to the traffic analysis for one or more of the following reasons:

1. Substantial design issues or constraints and the ability to satisfy California Highway Design Manual standards
2. Significant right-of-way (R/W) impacts
3. TAC / Team consensus, based on projected volume forecasts, that the concept would not meet the primary project objectives to improve managed lane connectivity or traffic operations

Table 4-2: Evaluated Geometric Improvement Concepts

| ID | Title | Description |
| :---: | :---: | :---: |
| A | Adaptive Ramp Metering | Adaptive ramp metering installed and activated at all onramps except EB SR 24 to NB I-680 |
| B | General Purpose (GP) Lane Conversion (South) | Convert GP Lane (\#1 lane) to express lane from Livorna Road On-Ramp to Ygnacio Valley off-ramp. |
| C | General Purpose (GP) <br> Lane Conversion (North) | Convert GP lane (\#1 lane) to express lane from North Main Street Overcrossing (O/C) to the start of planned express lane north of SR 242. |
| D | Contra Flow Lane (south of SR 24 to North Main St. O/C) | Near the northbound Rudgear Road on-ramp the northbound express lane would transition to the southbound I-680 express lane and operate as a contraflow lane roughly between the Rudgear on-ramp and North Main Street offramp. The southbound I-680 express lane would be inoperable between the North Main Street and Rudgear Road interchanges during the operation of the contra flow lane. |
| F1 | Collector-Distributor (C- <br> D) Road (North Main Street/Treat Boulevard Area) | Provide a C-D road to service the North Main Street offramp, North Main Street on-ramp, and Treat Boulevard offramp. The C-D road is intended to address the existing bottleneck between the North Main Street on-ramp and Treat Boulevard off-ramp by moving weaving traffic from the mainline to the C-D road. |
| J | Outside Widening <br> Livorna Road to Rudgear Road | Widen the mainline on the outside to provide a new lane from the Livorna Road on-ramp to the Rudgear Road onramp. With the widening a $5^{\text {th }}$ lane would be provided from the Livorna Road on-ramp to the Olympic/SR 24 off-ramp. |
| K | Outside Widening <br> Olympic Boulevard to <br> Ygnacio Valley Road | Widen the mainline on the outside from the Olympic/SR 24 off-ramp to the Olympic Boulevard on-ramp. With the widening a $4^{\text {th }}$ lane would be provided from the Olympic/SR 24 off-ramp to the Ygnacio Valley Road off-ramp. |
| M1 | Extend the Managed Lane from SR 242 South to North Main Street O/C (Plus) | Extend the northern planned managed lane (including new lane from SR 242 to North Main OC Geometric ID "L") to the south an additional 1,950 feet south of the North Main Street on-ramp. |
| N | SR 24/I-680 Interchange reconfiguration (Swap SR 24 and I-680 lanes including Ygnacio Boulevard off-ramp reconfiguration) | Reconfigure the SR 24/I-680 interchange such that eastbound SR 24 would join northbound I-680 on the right side instead of the left side. Provide a new direct connector to the Ygnacio Boulevard off-ramp from the existing northbound to westbound SR 24 flyover (the existing offramp from northbound I-680 to the Ygnacio Boulevard would be closed). |


| ID | Title | Description |
| :---: | :--- | :--- |
| O | Ygnacio Boulevard Off- <br> Ramp Reconfiguration | Provide a new direct connector to the Ygnacio Boulevard <br> SR 24 flyover (the existing offhbound I-680 to westbound <br> to the Ygnacio Boulevard off-ramp would be closed). |
| X | NB I-680 - Express Lane <br> Infrastructure | Provide Express Lane infrastructure (civil and TSI <br> components) for I-680 northbound from Livorna Road to 1- <br> mile south of the Benicia Bridge Toll Plaza (12 Miles) |

Table 4-3: Eliminated Geometric Improvement Concepts

| ID | Title | Description |
| :---: | :---: | :---: |
| E | Tunnel | Construct a tunnel underneath the I-680/SR 24 interchange to allow a continuous express lane through the interchange. Preliminary geometric review of the tunnel concept indicates that grades may need to be as high as $15 \%$. Due to the significant design issues associated with the concept, it was not progressed into the traffic analysis. |
| F2 | Braided Ramps (North Main Street/Treat Boulevard Area) | Provide braided ramps as an alternative to the C-D system. (F1).Due to the similar operational improvements and the significant increased cost compared with the C-D Road Concept (F1), this concept was not progressed into the traffic analysis. |
| G | North Main Street (Lawrence Way) On-Ramp Direct Connector | Provide a direct express lane connector to the median on northbound I-680 from Lawrence Way on-ramp (North Main St). It was determined that the direct connector would effectively prohibit the extension of the express lane to the south of the North Main Street and beginning the express lane at this location would not be effective in addressing the existing bottleneck between the North Main Street on-ramp and Treat Boulevard off-ramp. Due to these concerns the concept was not progressed into the traffic analysis. |
| H | Permanent <br> Northbound Express Lane on the Southbound Roadbed | Use the southbound roadbed (primarily southbound shoulder) to provide a northbound express lane. Currently (2016), there is sufficient shoulder width to consider this concept; however, the southbound HOV Completion project (to be completed by year 2020) will utilize the majority of the southbound shoulder to close the existing southbound HOV lane gap. The concept was not progressed into traffic analysis. |
| I | Shoulder Running Lane | Use the existing right-side shoulder to provide the necessary roadway width for a northbound express lane. Further review of this concept revealed that there are geometric, grade, and operational design challenges at each off- and on-ramp. Most of the constraints are in the vicinity of the SR 24 interchange. A shoulder running lane between I-580 and SR 24 for buses was evaluated as part of a CCTA effort entitled I-680 Transit Investment/Congestion Relief Options Study. The concept was not progressed into the traffic analysis. |


| ID | Title | Description |
| :---: | :---: | :---: |
| L | Extend the <br> Managed Lane <br> from SR 242 <br> South to North <br> Main Street O/C | This improvement is presented in the March 2007 Caltrans PSR. Further analysis of this concept by CCTA in 2014 revealed that the proposed start of the express lane would not allow sufficient distance for northbound drivers to transition from the general purpose lanes to the managed lane prior to reaching the bottleneck between the North Main Street on-ramp and Treat Boulevard offramp. Thus, minimal operational benefit would be achieved. The 2014 analysis indicated that the managed lane would need to start at least 1,500 feet prior to the North Main Street on-ramp to provide a substantive operational benefit (Concept M1 includes the additional 1,500 feet). Due to these findings the concept was not progressed into the traffic analysis. |
| M2 | Extend the Managed Lane from SR 242 South to SR 24 | Extend the northern managed lane to the south to about 4,860 feet south of the North Main Street on-ramp. Due to substantial ROW and design constraints the concept was not progressed into the traffic analysis. |

## 5. GEOMETRIC CONCEPTS EVALUATION

The design team evaluated each of the improvement concepts that were identified by the TAC to be progressed into the next phase of the study. The team performed a design evaluation of each of the concepts including considerations for construction costs, R/W impacts, design feasibility and constructability. Safety considerations were also noted in association with each concept's required exceptions to design standards from the California Highway Design Manual (HDM).

## Design Evaluation Criteria

Cost Range: Costs shown include capital construction costs for each of the geometric improvement concepts. Project development costs (including design and oversight), Environmental Mitigation and R/W costs are not included in the estimates. General quantities were prepared for the major work items including pavement widening, structure widening, retaining walls, sound walls and signing and striping. Lump sum estimates of work were prepared for grading and drainage, traffic handling and various miscellaneous construction items based off the size and scale of the overall construction work required for each of the concepts. The capital cost range includes a cost confidence factor and constructability factor as discussed below.

Cost Confidence: A cost confidence factor has been applied to account for uncertainty in construction cost due to complexity, constructability and uncertainty in the construction work associated with each geometric improvement concept. The cost confidence factor has been applied to the construction estimate values. A high cost confidence factor applies a $0 \%$ to $20 \%$
cost range above the base estimate, a medium cost confidence factor is $20 \%$ to $40 \%$, and a low cost confidence factor applies $40 \%$ to $60 \%$ range above the base estimate.

R/W Impact: R/W impacts have been evaluated for each of the geometric improvement concepts. A high impact value assumes that there is significant R/W acquisition required to construct the improvement concept that would substantially affect the project cost and delivery schedule. A medium impact value assumes there is some R/W acquisition required for construction but the affect is relatively lower to the project cost and delivery schedule. A low impact value assumes there is no R/W acquisition required for construction of the improvement concept. R/W impacts were based on design and R/W information available at the time of this study.

Design Feasibility: This factor is based on the magnitude, type and likelihood of obtaining the necessary exceptions to the HDM design standards associated with each geometric improvement concept. A high design feasibility value assumes low risk with approval of design exceptions. A low feasibility value is used when there are high risks associated with design approval from Caltrans.

Constructability: The constructability factor is used to account for the overall complexity of the construction involved for each design improvement concepts. A high constructability value is assigned to the concepts where the construction work is typical for highway construction, provides adequate work areas and would require little impact to existing traffic. A low value is assigned when the construction work is very complex, requires significant impacts to traffic and the work area is highly constrained.

Table 5-1 summarizes the evaluation of each geometric improvement concept and includes additional considerations associated with each improvement concept.

Table 5-1: Summary of Geometric Concepts Evaluation

| ID | Cost Range | Cost <br> Confidence | R/W Impact | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Adaptive Ramp Metering |  |  |  |  |  |
| A | \$15M to \$20M | High | Med | High | High |
| Design Summary | Seven (7) of the I-680 northbound on-ramps will have TOS elements installed for ramp metering. Design Exceptions: If R/W can not be acquired at certain ramps to accommodate HOV by-pass lanes a Caltrans policy exception would be required to install the ramp metering improvements without a HOV by-pass lane. |  |  |  |  |

[^0]| ID | Cost Range | Cost <br> Confidence | R/W Impact | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| General Purpose (GP) Lane Conversion (South) |  |  |  |  |  |
| B | \$900K to \$1.1M | High | Low | High | High |
| Design Summary | Converting a general purpose (GP) lane into a managed lane is relatively noncomplex. Work mainly includes re-striping and signing. There are about 3.9 miles of GP lane conversion along I-680 assumed for this concept (south of the SR 24 Interchange). Costs do not include express lane system costs. Design <br> Exceptions: Although it is assumed there are no new design exceptions required for this design concept, implementation of an express lane may require a striped 2foot wide buffer along the limits, potentially requiring new design exceptions to reduce shoulder width or lane width to accommodate the buffer. Conversion of a GP lane would require policy approval from FHWA and Caltrans. |  |  |  |  |
| Considerations | R/W: It is assumed that no new R/W would be required for this concept and use of the existing pavement width would be utilized. Constructability: Construction work is straight forward and would require re-striping on I-680 to be performed during night time hours. Safety: There should be little concern with safety for this concept, provided the design meets currents standards. |  |  |  |  |
| General Purpose (GP) Lane Conversion (North) |  |  |  |  |  |
| C | \$900K to \$1.1M | High | Low | High | High |
| Design Summary | Converting a general purpose (GP) lane into a managed lane is relatively noncomplex. Work mainly includes re-striping and signing. There are about 3.3 miles of GP lane conversion along I-680 assumed for this concept (north of the SR 24 Interchange). Costs do not include express lane system costs. Design Exceptions: Although it is assumed there are no new design exceptions required for this design concept, implementation of an express lane may require a striped 2 foot wide buffer along the limits, potentially requiring new design exceptions to reduce shoulder width or lane width to accommodate the buffer. Conversion of a GP lane would require policy approval from FHWA and Caltrans. |  |  |  |  |

R/W: It is assumed that no new R/W would be required for this concept and use of the existing pavement width would be utilized. Constructability: Construction work is non-complex and would require re-striping on I-680 to be performed during night time hours. Safety: Safety concerns are low for this concept, provided the design meets currents standards. There is a proposed left lane drop after the SR 24 merge and before the start of the managed lane. This proposed lane drop

## Considerations

 feature should be evaluated in the next phase of project design. Lane Conversion: This improvement assumes a GP lane drop before the start of the converted managed lane near N. Main St . During the next phase of design a direct GP lane to managed lane facility should be considered without a lane drop. Advanced signing can be installed per MUTCD to address the transition without a lane drop. The two options should be discussed with Caltrans and considered in the Traffic Operations Analysis during the next phase of design.| ID | Cost Range | Cost Confidence | R/W Impact | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Contra Flow Lane (south of SR 24 to North Main St. O/C) |  |  |  |  |  |
| D | \$20M to \$25M | Med | Low | Low | Hig |
| Design Summary | The Contra Flow Lane concept provides just over 2 miles of an I-680 northbound managed lane along the southbound travel way section. It requires a left exit to access the lane south of SR 24 and a merge section back to northbound I-680 near $N$. Main Street. The north merge location would require a left side lane drop along the southbound direction of travel while the contra flow lane is operational in the PM peak period. The concept would also require the use of a movable barrier system to protect the contra flow lane traffic during operations. The movable barrier system would include a "zipper" truck and a location to house and store the truck while not in use. The cost estimate assumes the costs for the movable barrier system and truck. There would be an on-going cost of about $\$ 400 \mathrm{~K} /$ year for operations and maintenance of the system. Design Exceptions: There are several design exceptions required for this concept. Reduction of the southbound left shoulder width would be required to store the moveable barrier while not in operation (shoulder reduction of 2 -feet). During operations there would be design exceptions associated with the travel lane widths (11-feet) and the proposed shoulder widths along the movable barrier limits ( $0-2$-foot shoulders). This concept would also require a left side lane drop along southbound I-680 during contra flow lane operation. |  |  |  |  |

R/W: It is assumed that no new R/W would be required for this concept and the existing pavement width would be utilized. Constructability: Construction work is non-complex and would require work to the median barrier for the northbound/southbound merge sections on I-680 to be performed during night time hours. Consideration of the overhead express lane signs would be required to

## Considerations

 allow for dual direction operation and could present issues with the tolling system design. Safety: Due to multiple new design exceptions, a southbound left side lane drop and introduction of a new managed lane concept to California, the safety considerations are high. Approval would be required from Caltrans and FHWA to implement the contra flow lane. A proposed 50 MPH speed limit for the contra flow lane should be discussed with Caltrans design and safety in PA/ED.| ID | Cost Range | Cost <br> Confidence | $\begin{aligned} & \text { R/W } \\ & \text { Impact } \end{aligned}$ | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Collector-Distributor (C-D) Road (North Main Street/Treat Boulevard Area) |  |  |  |  |  |
| F1 | \$47M to \$58M | Med | Med | Med | Med |
| Design Summary | The collector -distributor (C-D) road concept would provide a C-D system from the N . Main off-ramp to the Treat Blvd. on-ramp. The concept provides a two lane offramp at N . Main St, a three lane C-D roadway section between the Lawrence Way on-ramp and the Treat Blvd. off-ramp and a 1-lane on-ramp onto I-680 at the location of the current truck scale on-ramp. A 4,700-foot long retaining wall structure would be required along the outside of the proposed pavement widening to reduce R/W impacts and conflicts with BART. Structure widening would be required at the Contra Costa Canal bridge as well as the potential to addition of sound barriers along the outside of the C-D road on top of the proposed retaining structure. This concept would require the closure of the northbound CHP truck scale facility. A high speed Weigh-In-Motion (WIM) and Bypass system is assumed in the cost estimate to mitigate the closure of the Truck Scale Facility. Also $\$ 5 \mathrm{M}$ to $\$ 10 \mathrm{M}$ in costs has been included to mitigate operational conflicts with the City of Walnut Creek Maintenance Facility (see R/W impact below). Design Exceptions: There would be no new major design exceptions associated with this concept. There is a design constraint at the location of the N. Main St overcrossing structure that would require a design exception for left shoulder width and a special barrier design to protect the existing structure columns in order to maximize the travel way width for the proposed two lane off-ramp. |  |  |  |  |
| Considerations | R/W: This conce On the City of W existing set-back required from th setbacks. Addit impact at this loca would be off of the Main Street off-r likely that tempo work and could also be required work encroache constructed alon construction work feet) of BART op construction pha should be minim meets current st present any ope | t would requir Inut Creek pa of the building BART property nal design ex tion. Constr I-680 mainlin mp, Lawrence ry closures of done over a complete the onto city prop the existing R would require rating right-of . Safety: concern with andards and the tional issues. | W acqu there ucture. at does tions co ability: his conc y on-ra se ramp ay week ajority o and req line of ordinatio $y$ and cou $r$ than $c$ safety erge se | on on two pa d be potentia e would be ppear to imp be proposed ough most would temp and Treat BI ould be requir closure. Ni work. The s a retaining ans and the ith BART for increase the ruction phas is concept $p$ along the | ( 0.1 to 0.2 AC ). pacts to the mall R/W acquisition the BART line or reduce the R/W e construction work ily impact the N . ff-ramp. It is highly to complete the time closure would oosed widening ucture to be RT parcel. The rk in or near (25 ration of the nsiderations there ded the design road does not |


| ID | Cost Range | Cost <br> Confidence | R/W <br> Impact | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Outside Widening Livorna Road to Rudgear Road |  |  |  |  |  |
| J | \$25M to \$30M | High | Low | High | High |
| Design Summary | This concept would provide a 2-mile long additional northbound lane from the Livorna Road on-ramp to the existing auxiliary lane between the Rudgear Road on-ramp and the South Main Street off-ramp. This concept would provide a 5th lane from the Livorna Road on-ramp to the Olympic Blvd. off-ramp. It would require about 8,100 -feet ( 25 to 35 -foot max height) of retaining structures along the outside pavement widening limits. The concept would also require structure widening of the Rudgear Road Undercrossing (UC) structure. Design Exceptions: There are no new design exceptions assumed for this design concept. |  |  |  |  |
| Considerations | R/W: It is assumed that no new R/W would be required for this concept and the proposed outside retaining wall would minimize R/W impacts. Constructability: Construction work would require outside pavement widening and most of the work could be done without major impacts to existing traffic. The concept would require structure widening of the Rudgear Road UC and would temporarily impact local traffic on Rudgear Road to complete the work. The proposed retaining wall would likely be a soil nail type to allow for fewer impacts during construction. The overall wall height of the proposed retaining structure would need to be investigated to determine the best wall type along the proposed limits. Safety: There should be minimal concern with safety for this concept provided the design meets current standards. During the construction phase, protection of local traffic along Rudgear Road would need to be considered to accommodate the structure widening. |  |  |  |  |
| Outside Widening Olympic Boulevard to Ygnacio Valley Road |  |  |  |  |  |
| K | \$6.5M to \$8.0M | High | Low | High | High |
| Design Summary | This concept would provide a 0.6 -mile long additional lane from the SR 24 offramp to the Olympic Blvd on-ramp. This concept would provide a 4th lane from SR 24 off-ramp to the Ygnacio Valley Blvd off-ramp. It would require outside structure widening of the Olympic Blvd UC and Mount Diablo UC structures. The work would also include minor re-alignment of the Olympic Blvd on and off ramps. Design Exceptions: There are no new design exceptions assumed for this design concept. |  |  |  |  |



R/W: This concept would require new R/W acquisitions from two parcels (0.3 AC) to allow room to widen I-680 and realign Buskirk Ave. Also, significant utility relocation work would be required for this design concept. Constructability: Construction work would require structure replacement and structure widening,

## Considerations

 outside pavement widening, sound wall and retaining wall construction. The majority of this work would be performed during night time hours. Reconstruction of the Truck Scale facility and Buskirk Ave would require additional traffic control and detours. Safety: There are safety considerations that will need to be considered during the review and approval of the project in the PA/ED phase. There are also several safety considerations for the construction phase including overhead structure replacement at SR 242.| ID | Cost Range | Cost Confidence | R/W Impact | Design Feasibility | Constru |
| :---: | :---: | :---: | :---: | :---: | :---: |
| SR 24/I-680 Interchange reconfiguration (Swap SR 24 and I-680 lanes including Ygnacio Boulevard off-ramp reconfiguration) |  |  |  |  |  |
| N | $\begin{gathered} \text { \$500M to } \\ \$ 600 \mathrm{M} \end{gathered}$ | Low | High | Low | Low |
| Design Summary | This concept would realign the EB SR24 connector on-ramp to the right side (outside) of the I-680 travel way. This re-alignment would allow for an additional northbound managed lane to be constructed closing the existing managed lane gap through the SR 24/I-680 Interchange location. The design work would be significant and very complex. Major demolition work would be required with significant pavement widening and reconstruction. The concept would include about 7,600 of reconstructed concrete barrier, about 6,900-feet of reconstructed retaining walls ( 16 to 35 -feet in height), reconstruction of three major structures (SR24 WB Connector, SR 24 EB Connector and SR 24/Ygnacio Valley ramp), a new aerial structure for the I-680 Ygnacio Valley Blvd off-ramp (re-aligned to diverge off of the SR24 WB off-ramp connector) and outside structure widening of Ygnacio Valley UC and Parkside UC structures. Design Exceptions: Left and right shoulder width design exceptions would be required along I-680 at the existing BART and Trinity Ave OC structure columns (approximately 4-foot shoulder widths). |  |  |  |  |

## Considerations

R/W: This concept would require significant R/W acquisition on 16-parcels (about 1.0 AC total). The concept would result in impacts to 13 existing building structures within the acquired parcels and one parking lot area. The amount of R/W acquisition could increase if the remaining parcel size is determined to be unusable during the R/W appraisal evaluation process. Constructability: There are numerous concerns regarding the constructability of this design concept. The work is very complex and the added need to maintain existing traffic flows on I-680 and SR 24 may potentially make this concept unfeasible to construct. Significant detail is needed to determine the constructability and feasibility of this concept if it moves forward into a PA/ED phase for implementation. Safety: Due to the complex nature of this design concept various safety considerations should be reviewed during the PA/ED phase; construction phase safety is likely to be the biggest concern.

| ID | Cost Range | Cost <br> Confidence | R/W Impact | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Ygnacio Boulevard Off- Ramp Reconfiguration |  |  |  |  |  |
| 0 | \$90M to \$100M | Low | High | Med | Low |
| Design Summary | This concept would add a new aerial structure to serve the $\mathrm{I}-680$ northbound traffic to Ygnacio Valley Blvd via the existing SR 24 WB connector structure. Widening of the existing SR 24 WB connector would be required to provide a three lane diverging connector configuration from northbound I-680. The concept would re-route Ygnacio Valley Blvd users to the SR 24 off-ramp then to an exit ramp. The concept would also close the existing I-680 off-ramp to Ygnacio Blvd. The concept would require a 1,100-foot retaining wall along the approach to Ygnacio Valley Blvd. This design concept is also included under design concept N (SR 24/680 Interchange reconfiguration). Design Exceptions: There are no new design exceptions assumed for this design concept. |  |  |  |  |
| Considerations | R/W: This concept would require significant R/W acquisition on nine parcels (about 0.7 AC total). This concept would result in impacts to seven existing building structures within the acquired parcels. The amount of R/W acquisition could increase if the remaining parcel size is determined to be unusable during the R/W appraisal evaluation process. Constructability: If adequate R/W is acquired to provide for ample construction room, the construction work for this concept could be done without major concerns. Work would be required over Olympic and Mount Diablo Blvd impacting local traffic. Safety: Due to the complex nature of this design concept various safety considerations should be reviewed during the PA/ED phase; construction phase safety is likely to be the biggest concern. |  |  |  |  |
| NB I-680-Express Lane Infrastructure |  |  |  |  |  |
| X | \$18M to \$24M | High | Low | High | High |
| Design Summary | This concept northbound fro Costs include components. already have exceptions for for sign and ga foot wide buffer | uld provide t Livorna Roa all civil and Tol here are no b een installed in his concept w try structures (if required). | addition the Ben ystem Int haul cos e corrido include d lane wi | xpress lane Bridge Toll ation (TSI) c assumed (MT Design Exc duction of med s for the inst | structure on I-680 za (12 miles). ruction Backhaul system will ons: New design n shoulder widths tion of a striped 2- |


| ID | Cost Range | Cost Confidence | $\begin{aligned} & \text { R/W } \\ & \text { Impact } \end{aligned}$ | Design Feasibility | Constructability |
| :---: | :---: | :---: | :---: | :---: | :---: |
| Considerations | R/W: It is assumed that no new R/W will be required for this concept. Constructability: Construction work would mostly be performed at night and within the median of I-680 and along the outside shoulder areas. Temporary lane closures would be necessary to complete the work along with the potential for temporary lane shifts to provide an adequate working area in the median of I680. Safety: Due to the complex nature of this design concept with respect to traffic operations and weaving (buffer determination), several safety considerations would be reviewed during the PA/ED phase of the project and will be identified in the Traffic Safety Analysis Report (TSAR). |  |  |  |  |

## 6. ALTERNATIVES RECOMMENDED FOR TRAFFIC STUDY

Table 6-1 presents the traffic analysis alternatives that were evaluated in the second phase of this DAA study. The traffic analysis alternatives were developed based on combinations of the various geometric concepts presented above.

A total of ten alternatives (including No Build) were studied as part of this evaluation under year 2020 conditions. Due to design constraints along the corridor only two of the ten alternatives (Alternatives 8 and 9) would provide a continuous managed lane (eliminate the gap). Six of the alternatives would reduce the length of the gap (Alternatives 3, 4, 5, 6, 6A, and 7). Two alternatives (Alternative 1 and 2 ) would maintain the existing gap.

## Table 6-1: Traffic Analysis Alternatives

| Alternative | Kitle | This is the baseline alternative. Assumes that by year <br> 2020 the northbound I-680 Express Lane is operational <br> on the southern end of the study corridor. The <br> managed lanes gap is about 7.5 miles. |
| :---: | :---: | :--- |
| $\mathbf{1}$ | No Build | This alternative assumes adaptive ramp metering. This <br> is also included as part of Alternatives 3 through 9. |
| $\mathbf{2}$Adaptive Ramp <br> Metering | Express Lane: GP <br> Lane Conversion with <br>  <br> Ride Investments | This alternative converts a General Purpose (GP) Lane <br> to an Express Lane. This alternative assumes a 20\% <br> mode shift from SOV to HOV and transit, as a result of <br> an expanded Express Lane system and future corridor <br> investments in transit and park-n-ride facilities with |


|  |  | shuttle service to BART stations. This alternative also assumes a policy change for the HOV Lane from 2+ persons to $3+$ persons and an increase in HOV occupancy from an average of about 2.2 persons under existing conditions to 4.0 persons. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| :---: | :---: | :---: |
| 4 | Express Lane: GP <br> Lane Conversion Plus <br> C-D System with <br>  <br> Ride Investments | This alternative is similar to Alternative 3 but also provides a Collector-Distributor (C-D) road system to service the North Main Street off-ramp, North Main Street on-ramp, and Treat Boulevard off-ramp to eliminate mainline weaving and capacity issues at this location. This alternative also assumes the $20 \%$ mode shift from SOV to HOV and transit, as well as the HOV occupancy change included in Alternative 3. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 5 | Express Lane <br> Extension and GP <br> Lane Widening | This alternative provides outside widening on both the north side and south side of the SR 24 interchange to increase the length of the Express Lane and reduce the existing gap. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 6 | Express Lane <br> Extension and GP <br> Lane Widening Plus C-D System | This alternative is similar to Alternative 5 except it also provides a C-D road system to eliminate the mainline weaving between the North Main Street on-ramp and Treat Boulevard off-ramp. The managed lanes gap would be shortened from 7.5 miles to less than one mile. |
| 6A | Express Lane: GP <br> Lane Conversion and GP Lane Widening Plus C-D System | This scenario is similar to Alternative 6 except it does not include a managed lane extension from N. Main St to SR 242 and only includes an express lane conversion from Livorna Rd to SR 24. The managed lanes gap would be shorten from 7.5 miles to about 4.5 miles. This alternative would be an initial phase of Alternative 6. |


|  | Express Lane <br> Extension and GP <br> Lane Widening Plus <br> C-D System and <br> Ygnacio I/C <br> Reconfiguration | This alternative is similar to Alternative 6 except it does <br> not include the mainline widening between Olympic <br> Boulevard and Ygnacio Boulevard and instead <br> reconfigures the Ygnacio Boulevard off-ramp. The <br> managed lanes gap would be shortened from 7.5 miles <br> to less than one mile. |
| :--- | :--- | :--- |
| $\mathbf{8} \quad$Contra-Flow Plus <br> Express Lane <br> Extension and GP <br> Lane Widening | This alternative provides a contra flow lane (by using <br> the southbound express lane during the PM peak) to <br> provide a continuous Express Lane with no gap. |  |
| $\mathbf{9}$SR 24 and Ygnacio <br> I/C Reconfiguration <br> Plus Express Lane <br> Extension and GP <br> Lane Widening | This alternative would reconfigure the I-680/SR 24 <br> interchange (SR 24 would join I-680 on the right hand <br> side as opposed to the left-side) to provide a <br> continuous Express Lane with no gap. |  |

## 7. TRAFFIC FORECAST AND OPERATIONS ANALYSIS

## DATA COLLECTION

## Existing Demand Volumes and Travel Speeds

The existing PM peak period demand volumes (3 PM to 7 PM ) for northbound I-680 from Bollinger Canyon Road to Concord Avenue are presented in Appendix D. The existing demand volumes were based on data from the I-680 North Express Lane Conversion and I-680 South Express Lane Conversion projects, and a PEMS traffic count (March/April 2015) south of the Bollinger Canyon Road interchange.

Northbound I-680 travel speed data for the study corridor was obtained from the INRIX database. INRIX provides a traffic flow archive with the capability to access speeds reported at the segment level for specific days and times of day. INRIX data are gathered from a variety of sources, including in-vehicle GPS systems, mobile smart phones, and roadway sensors. INRIX provides a much larger data set than could be collected performing travel-time surveys utilizing the floating-car method and therefore is more comprehensive. INRIX speed data for April/May 2015 was used to establish a typical weekday condition. The observed INRIX speed data for the corridor is presented in Appendix B.

## Existing Northbound I-680 Bottleneck Locations and Queue Observations

Bottleneck locations and queue length estimates for the 2015 PM peak period were determined primarily from field observations and INRIX speed data.

HOV LANE
No bottlenecks or queuing were consistently observed on the HOV Lane. However, between the Crow Canyon Road and Livorna Road interchanges the HOV Lane does experience a speed reduction between 4:00 PM and 6:30 PM as a result of congestion on the adjacent General Purpose Lanes. Slowing in the northern end of the HOV Lane was also observed as vehicles approach queues in the General Purpose Lanes extending from a bottleneck between the Lawrence Way on-ramp and Treat Boulevard Off-ramp.

## GENERAL PURPOSE LANES

Table 7-1 summarizes the bottleneck and queuing observations on northbound I-680 during the 2015 weekday PM study period. Four bottlenecks develop on northbound I-680 during the PM study period. The bottlenecks for northbound I-680 during the PM study period are:

- Between the El Pintado Road on-ramp and Stone Valley Road off-ramp
- Between the Livorna Road on-ramp and Rudgear Road off-ramp
- Between the Ygnacio Valley Road off-ramp and North Main Street off-ramp
- Between the North Main Street on-ramp and Treat Boulevard off-ramp

Table 7-1: NB I-680 General Purpose Lane Bottlenecks - PM Study Period

| Time Period | Bottleneck Location | Controlling <br> or Hidden <br> Bottleneck | Causes of <br> Bottleneck | Approximate Queue <br> Length and Location <br> of End of Queue |
| :--- | :--- | :---: | :--- | :---: |
| 3 to 3:30 PM | Between the Livorna on-ramp <br> and Rudgear off-ramp | Controlling | Lane utilization <br> imbalance \& uphill <br> grade | 0.8 miles (Stone Valley <br> on-ramp) |
| 3:30 to 4 PM | Between the North Main on- <br> ramp and Treat off-ramp | Controlling | High mainline and <br> on-ramp/off-ramp <br> weaving volume | 3.0 miles (Rudgear <br> Road on-ramp) |
|  | Between the Ygnacio Valley <br> off-ramp and North Main <br> Street off-ramp | Hidden | Hidden within the <br> congestion of N. <br> Main St. bottleneck | n/a |
|  | Between the Livorna on-ramp <br> and Rudgear off-ramp | Controlling | Lane utilization <br> imbalance \& uphill <br> grade | 1.3 miles (Stone Valley <br> off-ramp) |
|  | Between the El Pintado on- <br> ramp and Stone Valley off- <br> ramp | Controlling | High mainline <br> volumes \& uphill <br> grade | 2.4 miles (Sycamore |
| Valley off-ramp) |  |  |  |  |

Table 7-1: NB I-680 General Purpose Lane Bottlenecks - PM Study Period

| Time Period | Bottleneck Location | Controlling or Hidden Bottleneck | Causes of Bottleneck | Approximate Queue Length and Location of End of Queue |
| :---: | :---: | :---: | :---: | :---: |
|  | ramp and Treat off-ramp |  | on-ramp/off-ramp weaving volume | Valley off-ramp) |
|  | Between the Ygnacio Valley off-ramp and North Main Street off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the Livorna on-ramp and Rudgear off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the El Pintado onramp and Stone Valley offramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
| 4:30 to 5 PM | Between the North Main onramp and Treat off-ramp | Controlling | High mainline and on-ramp/off-ramp weaving volume | 11.4 miles (Crow Canyon WB on-ramp) |
|  | Between the Ygnacio Valley off-ramp and North Main Street off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the Livorna on-ramp and Rudgear off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the El Pintado onramp and Stone Valley offramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
| 5 to 5:30 PM | Between the North Main onramp and Treat off-ramp | Controlling | High mainline and on-ramp/off-ramp weaving volume | 11.4 miles (Crow Canyon WB on-ramp) |
|  | Between the Ygnacio Valley off-ramp and North Main Street off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the Livorna on-ramp and Rudgear off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the El Pintado onramp and Stone Valley offramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
| 5:30 to 6 PM | Between the North Main onramp and Treat off-ramp | Controlling | High mainline and on-ramp/off-ramp weaving volume | 11.4 miles (Crow Canyon WB on-ramp) |
|  | Between the Ygnacio Valley off-ramp and North Main Street off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the Livorna on-ramp and Rudgear off-ramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |
|  | Between the El Pintado onramp and Stone Valley offramp | Hidden | Hidden within the congestion of N . Main St. bottleneck | n/a |

Table 7-1: NB I-680 General Purpose Lane Bottlenecks - PM Study Period

| Time Period | Bottleneck Location | Controlling <br> or Hidden <br> Bottleneck | Causes of <br> Bottleneck | Approximate Queue <br> Length and Location <br> of End of Queue |
| :--- | :--- | :---: | :--- | :---: |
| 6 to 6:30 PM | Between the North Main on- <br> ramp and Treat off-ramp | Controlling | High mainline and <br> on-ramp/off-ramp <br> weaving volume | 9.4 miles (Sycamore <br> Valley off-ramp) |
|  | Between the Ygnacio Valley <br> off-ramp and North Main <br> Street off-ramp | Hidden | Hidden within the <br> congestion of N. <br> Main St. bottleneck | $\mathrm{n} / \mathrm{a}$ |
|  | Between the Livorna on-ramp <br> and Rudgear off-ramp | Hidden | Hidden within the <br> congestion of N. <br> Main St. bottleneck | $\mathrm{n} / \mathrm{a}$ |
|  | Between the El Pintado on- <br> ramp and Stone Valley off- <br> ramp | Hidden | Hidden within the <br> congestion of N. <br> Main St. bottleneck | $\mathrm{n} / \mathrm{a}$ |
| 6:30 to 7 PM | Between the Ygnacio Valley <br> off-ramp and North Main <br> Street off-ramp | Controlling | Lane utilization and <br> mainline lane drop | 2.7 miles (Rudgear on- |
| ramp) |  |  |  |  |

## VISSIM MODEL DEVELOPMENT

The VISSIM microsimulation model developed as part of the I-680 North Express Lane Conversion and I-680 South Express Lane Conversion projects were updated reflect year 2015 traffic conditions. The existing (2015) observed and modeled speed contour maps for the General Purpose Lanes on northbound I-680 are presented in Appendix B. Bottleneck locations, congestion duration, maximum queues, and travel times were replicated adequately in the updated VISSIM model.

Table 7-2 presents the observed versus modeled travel time. In general, the simulated travel times are more conservative than the observed travel times.

Table 7-2: Observed versus Modeled Travel Time on Northbound I-680 ${ }^{1}$

| PM Peak Period | Observed (2015) Travel <br> Time (minutes) | Modeled Travel Time <br> (minutes) | \% Difference |
| :---: | :---: | :---: | :---: |
| $3: 00-3: 30$ | 22.6 | 22.5 | $-0.2 \%$ |
| $3: 30-4: 00$ | 32.3 | 40.2 | $24.5 \%$ |
| $4: 00-4: 30$ | 39.2 | 48.3 | $23.0 \%$ |
| $4: 30-5: 00$ | 49.8 | 48.6 | $-2.4 \%$ |
| $5: 00-5: 30$ | 57.0 | 52.7 | $-7.6 \%$ |
| $5: 30-6: 00$ | 55.5 | 51.2 | $-7.7 \%$ |
| $6: 00-6: 30$ | 36.1 | 41.6 | $15.3 \%$ |
| $6: 30-7: 00$ | 23.2 | 25.5 | $9.8 \%$ |

${ }^{1}$ Limits are from the Alcosta Boulevard on-ramp to the SR 4 off-ramp. Source: Fehr \& Peers and INRIX, 2015.

## EXISTING NETWORK MEASURES OF EFFECTIVENESS

Several Measures of Effectiveness (MOEs) computed with the VISSIM models are being used to quantify traffic operations for the project study area. Network MOEs are presented for the four-hour study period to provide a better understanding of overall traffic operations during the study period. Note that some MOEs (such as vehicle miles of travel) are presented for all origin/destination pairs while others (such as average travel time) are presented for just travel through the corridor between two discreet points. A distinction is made because some MOEs are most meaningful when the delays and traffic volumes from all on-ramps and off-ramps are considered while others are most meaningful when comparing only travel through the entire corridor on the freeway as experienced by users. The network MOEs can be particularly useful when comparing project alternatives by demonstrating the aggregate benefits of the project beyond a single peak hour. Table 7-3 presents the existing network MOEs.

## All Origin-Destination Pairs

- Vehicle Miles of Travel (VMT) \& Person Miles of Travel (PMT) - is a measure of the total vehicle (person) throughput of the study area taking into consideration the actual volume served versus the demand and the trip lengths.
- Vehicle Hours of Delay (VHD) \& Person Hours of Delay (PHD) - is a measure of the total delay incurred by all vehicles (persons) during the study period due to congestion.


## Travel Through the Corridor

- Average Travel Time - is a measure of the time taken by all vehicles (on average) to travel through the network i.e., between two discreet points during the study period. The travel time calculation considers the average delay, vehicle queues, and friction caused by merging vehicles.
- Average Travel Speed - is a measure of vehicle speeds in the network that travel between two discreet points during the study period. This measure depends both on the posted speed for a given link and the level of congestion.


## Lane-Mile Duration Index

The lane-mile duration index is another useful MOE when comparing alternatives especially if the location of the bottlenecks will differ among the alternatives. The lane-mile duration index is determined by summing the product of congested lane-miles and congestion duration for segments of roadway. For the purposes of this study a modified version of the lane-mile duration index will be used as follows ${ }^{1}$ :

Modified Lane-Mile Duration Index = (Congested miles * Congestion Duration) / (Total miles * 4 Hours)

[^1]The modified lane-mile duration index will provide results between 0 and 1.0 and will be useful in determining how overall congestion changes from one alternative to the next. A value of 0 indicates that no roadway segments on northbound I-680 between I-580 and SR 4 are congested (speeds less than 35 mph ) while a value of 1.0 would indicate that every roadway segment on northbound $\mathrm{I}-680$ is congested for the entire four hour peak period. Therefore, a lower index value would indicate less overall congestion.

Table 7-3: Existing NB I-680 PM Study Period Network Measures of Effectiveness

| Measure |  |
| :--- | :---: |
| All Origin-Destination Pairs | Value |
| Vehicle Miles of Travel (VMT) | 769,225 |
| Person Miles of Travel (PMT) | 919,517 |
| Vehicle Hours of Delay (VHD) in hours | 359 |
| Person Hours of Delay (PHD) in hours | 408 |
| Travel Through the Corridor (I-580 to SR 4) |  |
| Average Travel Time (minutes): SOV | 43.1 |
| Average Travel Speed (mph): SOV | 32 |
| Average Travel Time (minutes): HOV | 32.0 |
| Average Travel Speed (mph): HOV | 43 |
| Modified Lane-Mile Duration Index | 0.35 |

Notes
All origin-destination pairs consider all on- and off-ramps in the study network
Travel through the corridor includes only those vehicles that travel between the I-580 on-ramp and SR 4 off-ramp. Delay is calculated relative to 65 mph on freeways.
Source: Fehr \& Peers, 2016

## EXPRESS LANE ASSUMPTIONS AND TRAFFIC DEMAND FORECASTS

## Year 2020 Roadway Network Assumptions

The roadway network assumptions for each of the alternatives were presented earlier in Table 6-1 in this study. The Express Lane operational assumptions are:

- Alternative 1 and 2 - The Express Lane is operational in the southern part of the study corridor (Alcosta Boulevard to Livorna Road). Express lane operations are not provided in the northern part of the study corridor (north of SR 242).
- Alternatives 3 through 9 - Existing HOV lanes and proposed managed lanes within the study corridor will operate as Express Lanes.


## Year 2020 Traffic Demand Forecasts

The traffic demand forecasts for each of the alternatives are presented in Appendix B. The traffic demand forecasts are generally the same for Alternatives 1, 2, 5, 6, 6A, 7, 8, and 9 .

The traffic demand forecasts for Alternatives 3 and 4 differ from the other alternatives due to a $20 \%$ mode shift assumption (SOV to HOV). The overall vehicle demand volumes for Alternative

3 and 4 are lower (about 20\% lower) than the other alternatives due to implementation of these alternatives with future corridor investments in transit that increase the overall average vehicle occupancy along the corridor and park-n-ride facilities with shuttle service to BART stations. It is estimated that about 103 northbound bus trips with an average occupancy of 60 passengers per bus during the PM peak period would be necessary to achieve the $20 \%$ vehicle demand reduction. Alternatives 3 and 4 also assume a policy change on vehicle occupancy for the HOV Lane from 2 persons to $3+$ persons to gain free access to the Express Lane. In general, Alternatives 3 and 4 assume that the HOV occupancy will increase from an average of 2.2 persons under existing conditions to an average of 4.0 persons. While the vehicle demand forecasts may be different between the alternatives the same person demand is assumed for each of the alternatives.

## 2020 Corridor Operations

This section summarizes the corridor operations analysis for year 2020 applying the calibrated/validated VISSIM micro-simulation model developed under Existing Conditions. The speed contour maps for each of the alternatives are presented in Appendix D. EXHIBIT 7presents the modified lane-mile duration index by alternative.


EXHIBIT 7-1: Modified Lane-Mile Duration Index

## Bottleneck and Queue Characteristics

## ALTERNATIVE 1

Under Alternative 1 conditions (year 2020 baseline) the overall congestion is anticipated to worsen compared to existing conditions. The modified lane-mile duration index is anticipated to increase from 0.35 to 0.50 . Under Alternative 1 conditions, the following controlling bottlenecks would remain:

- Between the Livorna Road on-ramp and Rudgear Road off-ramp (controlling between 3:00 PM and 4:00 PM)
- Between the Ygnacio Valley Road off-ramp and North Main Street off-ramp (controlling between 3:00 PM and 3:30 PM and 6:30 PM to 7:00 PM)
- Between the North Main Street on-ramp and Treat Boulevard off-ramp (controlling between 3:30 PM and 6:30 PM)

Under Alternative 1 conditions the existing controlling bottleneck between the El Pintado Road on-ramp and Stone Valley Road off-ramp would no longer be a controlling bottleneck as it will be hidden within the congestion of downstream bottlenecks.

Under existing conditions the bottleneck between the Ygnacio Valley Road off-ramp and North Main Street off-ramp is present under one time period (6:30 PM to 7:00 PM) and under Alternative 1 conditions this bottleneck will be present under two time periods (between 3:00 PM and 3:30 PM and 6:30 PM to 7:00 PM). The bottleneck is a result of the mainline lane drop at the North Main Street off-ramp. Less than 400 vehicles per hour are served at the North Main Street off-ramp resulting in low utilization of the outside lane at the ramp exit. Based on the traffic analysis model, vehicles on the outside lane heading past the North Main Street off-ramp wait to move to the General Purpose Lanes until after the Ygnacio Valley Road off-ramp resulting in last minute lane changes that contribute to the bottleneck formation.

Under existing conditions the cumulative effect of the controlling bottlenecks resulted in a maximum queue that extended to about the Crow Canyon Road diagonal on-ramp. By year 2020, the cumulative effect of the controlling bottlenecks will result in a maximum vehicle queue that extends to about the Alcosta Boulevard off-ramp. This represents a queue length increase of about 4.6 miles compared to existing conditions.

Due to the projected traffic growth a new controlling bottleneck is anticipated to form between the Contra Costa Boulevard off-ramp and Monument Boulevard off-ramp from 5:30 PM to 6:30 PM. The bottleneck is a result of the mainline lane drop at the Monument Boulevard off-ramp. Less than 400 vehicles per hour are served at the Monument Boulevard off-ramp resulting in low utilization of the outside lane at the ramp exit. Based on the traffic analysis model, vehicles on the outside lane (auxiliary lane) heading past the Monument Boulevard off-ramp wait to move to the General Purpose Lanes until after the Contra Cost Boulevard off-ramp resulting in last minute lane changes that contribute to the bottleneck formation and lower vehicle speeds.

Typical speeds through a bottleneck are between 35 mph and 55 mph , the vehicle speeds at this bottleneck are anticipated to be between 20 mph and 30 mph . The queue from this bottleneck is anticipated to extend to about the Oak Road on-ramp ( 0.60 miles).

## ALTERNATIVE 2

The modified lane-mile duration index is anticipated to remain unchanged under Alternative 2 compared to Alternative 1. Alternative 2 is expected to have the same controlling bottleneck locations as Alternative 1. The additional vehicle throughput as a result of ramp metering at the bottleneck between the North Main Street on-ramp and Treat Boulevard off-ramp results in the following when compared to Alternative 1:

- Overall higher congested speeds (lower delay) south of the North Main Street on-ramp bottleneck. The higher speeds are a result of the ramp meters that break up the platoons of vehicles entering the freeway and results in smoother traffic flow at the on-ramps. Compared to Alternative 1, Alternative 2 would result in a travel time reduction for vehicles traveling through the corridor. Alternative 2 would result in a maximum travel time savings of about 14 minutes.
- Due to ramp metering, the throughput of the bottleneck north of the North Main Street on-ramp will increase and, as a result, increase traffic demand for the bottleneck between the Contra Costa Boulevard off-ramp and the Monument Boulevard off-ramp. Consequently, the bottleneck duration will increase from 1 hour to 2 hours and the maximum vehicle queue will grow from the Oak Road on-ramp to the truck scales on-ramp (increase of about 0.4 miles).


## ALTERNATIVE 3

The modified lane-mile duration index is anticipated to decrease from 0.5 under Alternative 1 to 0.14 under Alternative 3 . This represents a substantial reduction in overall congestion. The reduction in congestion is primarily attributed to the reduced vehicle demand assumed under Alternative 3. Under Alternative 3 there are three controlling bottlenecks and they are:

- Between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 4:00 PM)
- Between the Livorna Road on-ramp and Rudgear Road off-ramp (controlling between 3:30 PM and 4:00 PM)
- Between the North Main Street off-ramp and North Main Street on-ramp (location of General Purpose Lane conversion to Managed Lane; controlling between 3:00 PM and 7:00 PM)

Under Alternative 3 the major bottleneck would be between the North Main Street off-ramp and North Main Street on-ramp where the number of General Purpose Lanes would be reduced from five to four to allow for the restriping of the left most General Purpose Lane to a Managed Lane. The queue from this bottleneck would extend through the upstream bottleneck between the Livorna Road on-ramp and Rudgear Road off-ramp and result in maximum queues extending to the Livorna Road off-ramp ( 4.8 miles). The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the El Cerro Road on-ramp ( 0.5 miles).

The Alternative 3 analysis assumes that a General Purpose Lane is dropped (left side lane drop) prior to restriping one of the General Purpose Lanes to a Managed Lane. The 2014 California Manual of Uniform Traffic Control Devices (CA MUTCD) provides guidance on how a General Purpose Lane could feed directly into a managed lane through proper signing and striping and should be considered in future studies of this alternative. A General Purpose Lane feeding directly into a managed lane would avoid a left side lane drop and would provide a higher vehicle throughput. It is likely that both the congestion duration and maximum queue length would be reduced at the North Main Street off-ramp to North Main Street on-ramp bottleneck if the General Purpose Lane was designed to feed directly into the managed lane.

## ALTERNATIVE 4

The modified lane-mile duration index is estimated to be 0.01 under Alternative 4. This indicates that almost all of the congestion present under Alternative 1 would be eliminated under Alternative 4. The reduction in congestion is partially attributed to the reduced vehicle demand assumed under Alternative 4. Alternative 4 provides a C-D road system to service the North Main Street off-ramp, North Main Street on-ramp, and Treat Boulevard off-ramp. The C-D road system is intended to address the existing bottleneck between the North Main Street on-ramp and Treat Boulevard off-ramp by moving weaving traffic from the mainline to the C-D road system. Other operational benefits of the C-D road system that improve traffic operations are:

- Reduces the mainline vehicle demand between the North Main Street off-ramp and North Main Street on-ramp
- Improves the lane utilization approaching the North Main Street off-ramp; thereby, increasing the mainline throughput at this location.

Under Alternative 4 there are three controlling bottlenecks and they include:

- Between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 4:00 PM)
- Between the Livorna Road on-ramp and Rudgear Road off-ramp (controlling between 3:30 PM and 4:00 PM)
- Between the SR 242 off-ramp and Express Lane add about 900 feet south of the Willow Pass Road off-ramp (controlling between 4:30 PM and 5:30 PM)

The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the El Cerro Road on-ramp ( 0.5 miles). The maximum vehicle queue from the Livorna Road bottleneck would extend to the Livorna Road off-ramp ( 0.3 miles). The bottleneck between the SR 242 off-ramp and Express Lane add would develop as a result of reducing the number of General Purpose Lanes from three to two at this location to provide the Managed Lane. The queue from this bottleneck would extend to the Monument Boulevard on-ramp ( 0.68 miles).

## ALTERNATIVE 5

The modified lane-mile duration index is anticipated to decrease from 0.5 under Alternative 1 to 0.21 under Alternative 5. This represents a substantial reduction in overall congestion. Alternative 5 would eliminate three of the four controlling bottlenecks present under Alternative 1. Under this alternative the outside roadway widening north of the Livorna Road on-ramp would help eliminate the bottleneck between the Livorna Road on-ramp and Rudgear Road offramp. The extension of the northern managed lane to the south to about 1,950 prior to the North Main Street on-ramp would increase the capacity between the North Main Street on-ramp and the Treat Boulevard off-ramp and eliminate the following two other bottlenecks present under Alternative 1:

- Between the North Main Street on-ramp and Treat Boulevard off-ramp
- Between the Contra Costa Boulevard off-ramp and Monument Boulevard off-ramp

The only remaining controlling bottleneck from Alternative 1 is between the Ygnacio Valley Road off-ramp and North Main Street off-ramp (controlling between 3 PM and 7:00 PM). The elimination of the Alternative 1 bottlenecks would reduce queuing south of the SR 24 interchange and, as a result, a new controlling bottleneck, hidden under Alternative 1 conditions, would be revealed between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 6:00 PM).

The vehicle speeds between Ygnacio Valley Road off-ramp and North Main Street off-ramp are anticipated to be between 20 mph and 25 mph (lower than the typical speeds through a bottleneck). This is a location where there is low lane utilization, due to the mainline lane drop at the North Main Street off-ramp and less than 400 vehicles per hour using the off-ramp. Based on the traffic analysis model, vehicles on the outside lane heading past the North Main Street off-ramp wait to move to the General Purpose Lanes until after the Ygnacio Valley Road offramp resulting in last minute lane changes that contribute to the bottleneck formation and lower vehicle speeds.

Overall, the congestion under Alternative 5 would be substantially less than Alternative 1 as a result of eliminating three of the four controlling bottlenecks. The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the Sycamore Valley off-ramp ( 2.4 miles) while the maximum vehicle queue from the Ygnacio Valley Road bottleneck would extend to about the Stone Valley Road on-ramp ( 5.0 miles).

## ALTERNATIVE 6

Under Alternative 6 the modified lane-mile duration index would decrease from 0.5 under Alternative 1 to 0.06 . This represents a substantial reduction in overall congestion. Alternative 6 includes the same improvements as Alternative 5 and as a result would eliminate the same bottlenecks Alternative 1. Similar to Alternative 5 the only remaining controlling bottleneck from Alternative 1 is between the Ygnacio Valley Road off-ramp and North Main Street off-ramp (controlling between 3 PM and 7:00 PM). However, the reduced queuing south of the SR 24
interchange associated with this alternative again reveals a new controlling bottleneck, that had been hidden in queues from downstream bottlenecks under Alternative 1 conditions, between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 6:00 PM).

Alternative 6 also includes the C-D roadway system which improves the lane utilization approaching the North Main Street off-ramp and as a result would substantially reduce the vehicle queues associated with the Ygnacio Valley Road off-ramp to North Main Street off-ramp bottleneck. The maximum vehicle queue from this bottleneck would extend to about the Olympic Boulevard on-ramp ( 0.2 miles). The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the Sycamore Valley off-ramp ( 2.4 miles). Overall, the congestion under Alternative 6 would be substantially less than Alternative 1 as a result of eliminating three of the four controlling bottlenecks and substantially reducing the congestion approaching the Ygnacio Valley Road bottleneck.

## ALTERNATIVE 6A

Under Alternative 6A the modified lane-mile duration index would decrease from 0.5 under Alternative 1 to 0.26 . This represents a substantial reduction in overall congestion. Alternative 6 A includes the same improvements as Alternative 6, except the extension of the northern managed lane south from SR 242 to the North Main Street interchange. The following controlling bottlenecks would develop under Alternative 6A:

- Between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 5:30 PM)
- Between the Ygnacio Valley Road off-ramp and North Main Street off-ramp (controlling between 3:00 PM and 3:30 PM)
- Between the Buskirk on-ramp and Oak Road on-ramp (controlling between 3:30 PM and 4:00 PM)
- Between the Contra Costa Boulevard off-ramp to Monument Boulevard off-ramp (controlling between 3:30 PM and 7:00 PM)

The primary difference between Alternative 6A and 6 is the controlling bottleneck between the Contra Costa Boulevard off-ramp and Monument Boulevard off-ramp that is anticipated under Alternative 6A. This is the result of not extending the northern managed lane to the south. Under Alternative 6A the queue from this bottleneck would extend through the upstream bottleneck at the El Pintado Road on-ramp at times during the peak period.

## ALTERNATIVE 7

Under Alternative 7 the modified lane-mile duration index would decrease from 0.5 under Alternative 1 to 0.06 . This represents a substantial reduction in overall congestion. Alternative 7 would close the existing Ygnacio Valley Road off-ramp and provide access to Ygnacio Valley Road via the SR 24 off-ramp. This design would reduce traffic demand for freeway sections
immediately north of the SR 24 off-ramp and, as a result, eliminate the need for widening between the SR 24 off-ramp and Olympic on-ramp (as in Alternative 6). Analysis results for this alternative indicate that bottleneck locations, congestion duration and queue lengths would be nearly identical to those for Alternative 6.

## ALTERNATIVE 8

Under Alternative 8 the modified lane-mile duration index would decrease from 0.5 under Alternative 1 to 0.06 . This represents a substantial reduction in overall congestion. Alternative 8 would eliminate the gap in the managed lane system by providing a contra flow lane near the I680/SR 24 interchange area to provide a continuous northbound Express Lane. This alternative would eliminate the following three controlling bottlenecks present under Alternative 1 conditions:

- Between the Livorna Road on-ramp and Rudgear Road off-ramp
- Between the Ygnacio Valley Road off-ramp and North Main Street off-ramp
- Between the Contra Costa Boulevard off-ramp and Monument Boulevard off-ramp

The only remaining controlling bottleneck from Alternative 1 is between the North Main Street on-ramp and Treat Boulevard off-ramp (between 3:30 PM and 4:00 PM). Similar to other alternatives which reduce queuing south of the SR 24 interchange, this alternative reveals a new controlling bottleneck between the El Pintado Road on-ramp and Stone Valley Road offramp (controlling between 3:30 PM and 6:00 PM).

The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the Sycamore Valley Road off-ramp ( 2.4 miles) and the maximum vehicle queue from the North Main Street bottleneck would extend to about the Ygnacio Valley Road on-ramp (1.1 miles). Overall, the congestion under Alternative 8 would be substantially less than Alternative 1 as a result of eliminating three of the four controlling bottlenecks and substantially reducing the congestion at the North Main Street on-ramp to Treat Boulevard off-ramp bottleneck.

A major drawback of this alternative is that the I-680 southbound Express Lane would be inoperable between the North Main Street and Rudgear Road interchanges so that the lane can be used as the northbound Express Lane. Some preliminary volume to capacity analysis was performed for the southbound I-680 direction to determine the potential impact of reducing the capacity for southbound traffic. The preliminary analysis indicates that within the contra-flow lane freeway section southbound traffic for year 2020 will not result in any new bottlenecks. As a result, no substantial change in congestion length or duration is anticipated for southbound traffic.

## ALTERNATIVE 9

Under Alternative 9 the modified lane-mile duration index would decrease from 0.5 under Alternative 1 to 0.06 . This represents a substantial reduction in overall congestion. Alternative 9 would reconfigure the I-680/SR 24 interchange to help eliminate the gap in the managed lane
system and provide a continuous northbound Express Lane. This alternative would provide about the same northbound capacity as Alternative 8 and as a result the analysis results are nearly identical to Alternative 8. This alternative would eliminate the following three controlling bottlenecks present under Alternative 1 conditions:

- Between the Livorna Road on-ramp and Rudgear Road off-ramp
- Between the Ygnacio Valley Road off-ramp and North Main Street off-ramp
- Between the Contra Costa Boulevard off-ramp and Monument Boulevard off-ramp

The only remaining controlling bottleneck from Alternative 1 is between the North Main Street on-ramp and Treat Boulevard off-ramp (between 3:30 PM and 4:00 PM). Similar to other alternatives this alternative reveals a new controlling bottleneck between the El Pintado Road on-ramp and Stone Valley Road off-ramp (controlling between 3:30 PM and 6:00 PM) as a result of eliminating several downstream controlling bottlenecks and reducing queues south of SR 24.

The maximum vehicle queue from the El Pintado Road bottleneck would extend to about the Sycamore Valley off-ramp ( 2.4 miles) and the maximum vehicle queue from the North Main Street bottleneck would extend to about the Ygnacio Valley Road on-ramp (1.1 miles). Overall, the congestion under Alternative 9 would be substantially less than Alternative 1 as a result of eliminating three of the four controlling bottlenecks and substantially reducing the congestion at the North Main Street on-ramp to Treat Boulevard off-ramp bottleneck.

## NETWORK MEASURES OF EFFECTIVENESS

Table 7-4 presents the network measures of effectiveness for all of the alternatives in year 2020. The percent change for Alternatives 2 through 9 compared to Alternative 1 is presented in parenthesis. ${ }^{2}$ The most informative MOEs with respect to comparing the alternatives are vehicle and person hours of delay and travel time. All of the alternatives reduce the vehicle and person hours of delay and reduce travel times (increase travel speeds) compared to Alternative 1.

EXHIBIT 7- presents a comparison of vehicle and person hours of by alternative. EXHIBIT 7presents a travel time comparison for SOVs by alternative while EXHIBIT 7- presents a travel time comparison for HOVs by alternative.

[^2]Table 7-4: Year 2020 Northbound I-680 PM Study Period Network Measures of Effectiveness

| Measure |  | Alternative |  |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| All OriginDestination Pairs | 1 | $2$ | 3 | 4 | 5 | 6 | 6A | 7 | 8 | 9 |
| Vehicle Miles of Travel (VMT) | 764,371 |  | 632,370 | 644,044 | 803,961 | 806,257 | 799,298 | 805,146 | 806,333 | 805,233 |
|  |  | (2\%) | (-17\%) | (-16\%) | (5\%) | (5\%) | (5\%) | (5\%) | (5\%) | (5\%) |
| Person Miles of Travel (PMT) | 907,985 | 922,965 | 962,995 | 963,800 | 958,526 | 963,619 | 953,770 | 962,261 | 963,746 | 962,354 |
|  |  | (2\%) | (6\%) | (6\%) | (6\%) | (6\%) | (5\%) | (6\%) | (6\%) | (6\%) |
| Vehicle Hours of Delay (VHD) in hours | 2,253 | 1,706 | 471 | 9 | 302 | 72 | 555 | 71 | 69 | 68 |
|  |  | (-24\%) | (-79\%) | (-99\%) | (-87\%) | (-97\%) | (-75\%) | (-97\%) | (-97\%) | (-97\%) |
| Person Hours of Delay (PHD) in hours | 2,460 | 1,859 | 510 | 11 | 316 | 76 | 595 | 75 | 71 | 71 |
|  |  | (-24\%) | (-79\%) | (-99\%) | (-87\%) | (-97\%) | (-76\%) | (-97\%) | (-97\%) | (-97\%) |
| Travel Through the Corridor (1-580 to SR 4) |  |  |  |  |  |  |  |  |  |  |
| Average Travel | 80.4 | 72.6 | 31.8 | 22.4 | 37.5 | 26.0 | 41.1 | 25.9 | 26.0 | 25.6 |
| Time (minutes): SOV |  | (-10\%) | (-60\%) | (-72\%) | (-53\%) | (-68\%) | (-49\%) | (-68\%) | (-68\%) | (-68\%) |
| Average Travel | 17.0 | 18.8 | 42.9 | 61.0 | 36.4 | 52.5 | 33.3 | 52.7 | 52.6 | 53.3 |
| Speed (mph): SOV |  | (11\%) | (153\%) | (259\%) | (114\%) | (209\%) | (96\%) | (210\%) | (210\%) | (214\%) |
| Average Travel | 40.6 | 38.3 | 26.2 | 22.1 | 25.3 | 22.2 | 29.9 | 22.2 | 22.0 | 22.0 |
| Time (minutes): HOV |  | (-6\%) | (-36\%) | (-46\%) | (-38\%) | (-45\%) | (-26\%) | (-45\%) | (-46\%) | (-46\%) |
| Average Travel | 33.7 | 35.6 | 52.2 | 61.8 | 54.0 | 61.4 | 45.7 | 61.5 | 62.1 | 62.1 |
| Speed (mph): HOV |  | (6\%) | (55\%) | (84\%) | (61\%) | (83\%) | (36\%) | (83\%) | (84\%) | (85\%) |

Notes:

1. Alternatives 3 and 4 assume a $20 \%$ reduction in vehicle demand as a result of mode shift.
2. Results presented in parenthesis show the percent change for the alternative compared to Alternative 1.


EXHIBIT 7-2: Northbound I-680 Vehicle and Person Hours of Delay by Alternative


EXHIBIT 7-3: Northbound I-680 SOV Travel Time by Alternative


EXHIBIT 7-4: Northbound I-680 HOV Travel Time by Alternative

## ALTERNATIVE 1

Existing network measures of effectiveness were presented in Table 7-3 while year 2020 network measures of effectiveness are presented in Table 7-4. Increased corridor traffic demand between 2015 and 2020 will result in Alternative 1 vehicle hours of delay growing from 359 to 2,253 ( $528 \%$ increase) while the person hours of delay will grow from 408 to 2,460 ( $503 \%$ increase). The average SOV travel time will grow from 43.1 minutes under existing conditions to 80.4 minutes ( $87 \%$ increase) while the average HOV travel time will grow from 32.0 minutes under existing conditions to 40.6 minutes ( $27 \%$ increase).

## ALTERNATIVE 2

Alternative 2 (with implementation of adaptive ramp metering) would reduce vehicle and person hours of delay by about $24 \%$ compared to Alternative 1. SOV and HOV average travel times are also anticipated to improve under Alternative 2 ( $10 \%$ reduction for SOVs and 6\% reduction for HOVs).

## ALTERNATIVE 3

Alternative 3 would reduce vehicle and person hours of delay compared to Alternative 1 ( $79 \%$ reduction for VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 3 ( $60 \%$ reduction for SOVs and $36 \%$ reduction for HOVs).

As discussed earlier, the Alternative 3 analysis assumes that a General Purpose Lane is dropped (left side lane drop) prior to restriping one of the General Purpose Lanes to a managed lane. It is anticipated that the lane drop would result in increased congestion (VHD and PHD) on eastbound SR 24 including the freeway connector when compared to Alternative 1 and the other alternatives. The left side lane drop would effectively reduce the number of eastbound SR 24 lanes that merge with northbound I-680 from three to two. The increased congestion on the SR 24 freeway connector is the reason why the VHD and PHD is higher for Alternative 3 than other alternatives that have a higher the travel time on northbound I-680 compared to Alternative 3. As discussed earlier, future studies that include this alternative should consider a design that has the General Purpose Lane feeding directly into the managed lane. This would reduce the congestion on eastbound SR 24.

## ALTERNATIVE 4

Alternative 4 would reduce vehicle and person hours of delay compared to Alternative 1 (99\% reduction for VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 4 ( $72 \%$ reduction for SOVs and $46 \%$ reduction for HOVs).

## ALTERNATIVE 5

Alternative 5 would reduce vehicle and person hours of delay compared to Alternative 1 ( $87 \%$ reduction for VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 5 ( $53 \%$ reduction for SOVs and $38 \%$ reduction for HOVs).

## ALTERNATIVE 6

Alternative 6 would reduce vehicle and person hours of delay compared to Alternative 1 ( $36 \%$ reduction for VHD and $39 \%$ reduction for PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 6 ( $68 \%$ reduction for SOVs and 45\% reduction for HOVs).

## ALTERNATIVE 7

Alternative 7 would reduce vehicle and person hours of delay compared to Alternative 1 ( $97 \%$ reduction for both VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 7 (68\% reduction for SOVs and 45\% reduction for HOVs).

## ALTERNATIVE 8

Alternative 8 would reduce vehicle and person hours of delay compared to Alternative 1 ( $97 \%$ reduction for both VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 8 ( $68 \%$ reduction for SOVs and $46 \%$ reduction for HOVs).

## ALTERNATIVE 9

Alternative 9 would reduce vehicle and person hours of delay compared to Alternative 1 ( $97 \%$ reduction for both VHD and PHD). SOV and HOV average travel times are also anticipated to improve under Alternative 9 ( $68 \%$ reduction for SOVs and $46 \%$ reduction for HOVs).

## 8. DESIGN ALTERNATIVE ASSESSMENT

A total of ten alternatives (including No Build) were studied as part of this evaluation under year 2020 conditions. Due to design constraints along the corridor only two of the ten alternatives (Alternatives 8 and 9) would provide a continuous managed lane (eliminate the gap). Six of the alternatives would reduce the length of the gap (Alternatives 3, 4, 5, 6, 6A, and 7). Two alternatives (Alternative 1 and 2) would maintain the existing gap. The DAA team has evaluated construction costs, operational improvements and associated risks for the proposed alternatives. The assessment of the proposed alternatives is provided below.

Table 8-1 ranks the studied traffic analysis alternatives by overall operational benefit for I680 NB congestion relief. The table includes the key geometric features associated with each alternative to illustrate common and unique features of each alternative for comparison purposes.

Table 8-1: Operational Improvement Ranking of Studied Alternatives

| RANK | ALTERNATIVE | DESIGN CONCEPTS | KEY DESIGN ELEMENT | $\begin{gathered} \text { COST } \\ \text { RANGE* } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: |
| 1 | Alternative 4 | A, B, C, F1, X | C-D Road w/ GP Conversion (Mode Shift) | \$85M - \$105M |
| 2 | Alternative 6 | A, B, F1, J, K, M1, X | C-D Road w/ Lane Extension | \$250M - \$300M |
|  | Alternative 8 | A, B, D, J, M1, X | Contra Flow w/ Lane Extension | \$220M - \$260M |
|  | Alternative 7 | A, B, F1, J, M1, O, X | Ygnacio Off-Ramp Configuration | \$350M - \$400M |
|  | Alternative 9 | A, B, J, K, M1, N, X | I680/ SR24 Interchange Reconfiguration | \$700M - \$900M |
| 6 | Alternative 5 | A, B, J, K, M1, X | Lane Extension | \$210M - \$250M |
| 7 | Alternative 3 | A, B, C, X | GP Conversion (Mode Shift) | \$35M - \$47M |
| 8 | Alternative 6A | A, B, F1, J, K, X | C-D Road | \$100M - \$125M |
| 9 | Alternative 2 | A | Ramp Metering | \$15M - \$20M |
| 10 | Alternative 1 | - | - | No Build |

[^3]Table 8-2 shows the studied alternatives and the associated benefit-cost ratio based on estimated average construction costs and an operational benefit factor based on the traffic analysis results.

Table 8-2: Alternative Cost-Benefit Ratios

| ALTERNATIVE | AVERAGE COST* <br> (MILLIONS \$) | OPERATIONAL <br> BENEFIT FACTOR | BENEFIT- <br> COST RATIO | BENEFIT- <br> COST <br> RANK |
| :---: | :---: | :---: | :---: | :---: |
| Alternative 2 | 17.5 | 240 | 13.7 | $\mathbf{1}$ |
| Alternative 3 | 90 | 790 | 8.8 | $\mathbf{2}$ |
| Alternative 4 | 144 | 999 | 6.9 | $\mathbf{3}$ |
| Alternative 6A | 112.5 | 760 | 6.8 | $\mathbf{4}$ |
| Alternative 8 | 240 | 970 | 4.0 | $\mathbf{5}$ |
| Alternative 5 | 230 | 870 | 3.8 | $\mathbf{6}$ |
| Alternative 6 | 275 | 970 | 3.5 | $\mathbf{7}$ |
| Alternative 7 | 375 | 970 | 2.6 | $\mathbf{8}$ |
| Alternative 9 | 800 | 970 | 1.2 | $\mathbf{9}$ |

* Costs shown are construction estimates only and do not include support costs


## 9. CONCLUSION AND RECOMENDATIONS

Based on the results presented in Section 8 the following conclusions can be made:

- Alternative 2 has the highest benefit-cost ratio as a result of a modest operational benefit but with a substantially lower cost compared to the other alternatives. Some key conclusions are :
o This alternative is included in all the other studied alternatives, except Alternative 1.
o Adaptive ramp metering if desired can be separated and delivered as an initial delivery project for NB I-680.
- Alternatives 4, 6 and 8 all provide similar substantial operational benefits for NB I680 under a different set of key assumptions and/or geometric improvement concepts:
o Alternative 4 assumes a $20 \%$ mode shift (SOV to HOV and transit, through investments in transit and park \& ride services) with the GP Lane Conversion Design Concept C (North Main to SR 242). It has a lower cost compared to Alternatives 6 and 8.
o Alternatives 4 and 6 both include a C-D Road (North Main to Treat).
o Alternative 6 and 8 include the Managed Lane Extension (North Main to SR 242).
o Alternative 8 includes a Contra Flow Lane and has a lower cost than Alternative 9 (the other alternative that closely the gap completely).
o Based on these findings and comparison to the other remaining alternatives these three alternatives are ideal for further study.
- Alternative 3 can be considered a subset (or an initial phase) of Alternative 4 as it also assumes a $20 \%$ mode shift and includes the same geometric improvement concepts except the C-D Road (North Main to Treat). Some key conclusions for Alternative 3 compared to Alternative 4
o Alternative 3 has a higher benefit-cost ratio than Alternative 4.
o Although Alternative 3 has a higher benefit-cost ratio, Alternative 4 is better suited for further study as an analysis can be performed with and without the C-D Road and effectively evaluate Alternative 3 and 4.
- Alternatives 5 and $6 A$ can be considered subsets (or an initial phases) of Alternative 6 due to the following:
o Alternative 5 includes all of the geometric improvement concepts as Alternative 6 except the C-D Road (North Main to Treat).
o Alternative 6A includes all of the geometric improvement concepts as Alternative 6 except the Managed Lane Extension (North Main to SR 242).
o Alternative 6 is better suited for further study vs. Alternatives 5 and 6A as Alternative 6 can be evaluated with and without the C-D road and Managed Lane Extension and effectively evaluate Alternatives 5, 6A, and 6.
- Alternatives 8 and 9 provide improvements that close the I-680 NB managed lane gap:
o Alternative 8 is better suited for further study compared to Alternative 9 as it fully meets the project objective (closing the gap in the managed lane) at a lower cost.
o Alternative 9 has the lowest benefit-cost ratio, highest cost, and substantial risks and challenges that make this alternative not suitable for further study.
- Alternatives 7 and 9 have the lowest benefit-cost ratios along with several risks that include:
o Significant project delivery risks and construction challenges.
o Alternatives 7 and 9 are not ideal alternatives to be carried through for further study due to the costs, risks and approval challenges.


## RECOMMENDED ALTERNATIVES

This DAA recommends three proposed alternatives be studied and compared to the No Build alternative to identify the preferred alternative during the next project delivery phase:

- No Build
- Alternative 4-GP Lane Conversion Plus C-D System with Transit and Park \& Ride Investments
- Alternative 6 - Express Lane Extension and GP Lane Widening Plus C-D System Alternative 8 - Contra-Flow Plus Express Lane Extension and GP Lane Widening


## OTHER CONSIDERATIONS

- Alternative 6 can serve as the baseline proposed alternative for the next phase:
o Alternative 6 has the largest impact footprint of the three proposed alternatives and includes almost all the associated environmental impacts of Alternatives 4 and 8.
o Alternative 6 can be evaluated for separate optional future phases of construction.
- Alternative 6A can be an initial construction phase and later include Design Concept of M1, Managed Lane Extension (North Main to SR 242) as a future construction phase to produce Alternative 6.
- Design Concept C, GP Lane Conversion Design (North Main to SR 242), included in Alternative 4 can be considered as a future construction phase.
o Alternative 6 includes more typical highway improvements compared with Alternatives 4 and 8 with less project approval risks.
- Alternative 8 is comparable to Alterative 6 for construction cost and operational benefit and is the only recommended alternative that closes the I-680 NB gap.
- Alternative 4 provides the highest operational benefits and lowest cost with the highest benefit-cost ratio of the three recommended alternatives.
- The GP Lane conversion element (Concept C) of Alternative 4 and Contra Flow Lane element (Concept D) of Alternative 8 should be vetted early during the next phase to identify delivery feasibility and Caltrans Approval as well as consider other associated project risks.


## APPENDIX A - Alternative Lane Diagrams

## LEGEND

------- SOUTHBOUND EXPRESS LANE
_ SOUTHBOUND EXPRESS LANE-RESTRICTED
.------- EXISTING NORTHBOUND EXPRESS LANE
PROPOSED ALTERNATIVE FEATURE
PROPOSED NORTHBOUND EXPRESS LANE
GENERAL PURPOSE LANE CONVERSION


$$
\text { NORTHBOUND } \longrightarrow
$$



I-680 NORTHBOUND

= SOUTHBOUND EXPRESS LANE-RESTRICTED
EXISTING NORTHBOUND EXPRESS LANE
PROPOSED ALTERNATIVE FEATURE
PROPOSED NORTHBOUND EXPRESS LANE
GENERAL PURPOSE LANE CONVERSION

 authority
$\longmapsto \vdash$ $\qquad$




------- SOUTHBOUND EXPRESS LANE
=_ SOUTHBOUND EXPRESS LANE-RESTRICTED
.------- EXISTING NORTHBOUND EXPRESS LANE
PROPOSED ALTERNATIVE FEATURE
PROPOSED NORTHBOUND EXPRESS LANE
GENERAL PURPOSE LANE CONVERSION


GEOMETRIC IMPROVEMENT CONCEPTS:

A RAMP METERING
GENERAL PURPOSE LANE
CONVERSION - SOUTH
F1 COLLECTOR DISTRIBUTOR ROAD
(w/ WIM AND BYPASS SYSTEM)

LANE ADDITION - LIVORNA RD TO RUDGEAR RD
K LANE ADDITION - SR24 TO
K OLYMPICBLVD
X EXPRESS LANE INFRASTRUCTURE (LIVORNA RD TO SR 24)

$\mathrm{Mr}^{T}$

$\longmapsto \supseteq$

Express Lane GP Lane Widening Plus C-D System Capital Construction Cost Range: $\$ 100 \mathrm{M}$ to $\$ 125 \mathrm{M}$ (Costs shown are construction estimates only and do not include R/W or support costs)

## I-680 NORTHBOUND DESIGN ALTERNATIVE ANALYSIS

 ALTERNATIVE 6A

## LEGEND:

------- SOUTHBOUND EXPRESS LANE
_ SOUTHBOUND EXPRESS LANE-RESTRICTED
.------- EXISTING NORTHBOUND EXPRESS LANE
PROPOSED ALTERNATIVE FEATURE
PROPOSED NORTHBOUND EXPRESS LANE
GENERAL PURPOSE LANE CONVERSION


## GEOMETRIC IMPROVEMENT CONCEPTS:

| A | RAMP METERING | J | LANE ADDITION - LIVORNA RD RUDGEAR RD |
| :---: | :---: | :---: | :---: |
| B | GENERAL PURPOSE LANE CONVERSION - SOUTH | M1 | EXTEND EXPRESS LANE FROM SR242 TO NORTH MAIN ST |
| F1 | COLLECTOR DISTRIBUTOR ROAD (w/ WIM AND BYPASS SYSTEM) | 0 | YGNACIO OFF RAMP RECONFIGURATION |
|  |  | X | EXPRESS LANE INFRASTRUCTU (LIVORNA RD TO TOLL PLAZA) |

RUDGEARRD
M1 EXTEND EXPRESS LANE FROM

- YGNACIO OFF RAMP

X $\begin{aligned} & \text { EXPRESS LANE INFRASTRUCTURE } \\ & \text { (LIVORNA RD TOTOLL PLAZA) }\end{aligned}$


$$
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$$




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ト?
Express Lane Extension and GP Lane Widening Plus C-D System and Ygnacio I/C Reconfiguration Capital Construction Cost Range: $\$ 350 \mathrm{M}$ to $\$ 400 \mathrm{M}$

I-680 NORTHBOUND DESIGN ALTERNATIVE ANALYSIS ALTERNATIVE 7




## APPENDIX B - Traffic Analysis Data

## EXISTING PM PEAK PERIOD DEMAND VOLUMES

|  | 2015 (Existing) Total Demand Volumes |  |  |  | 2015 (Existing) HOV Demand Volumes |  |  |  | 2015 (Existing) SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 6,021 | 5,931 | 6,027 | 5,660 | 705 | 860 | 835 | 729 | 5,316 | 5,071 | 5,192 | 4,931 |
| Bollinger Off-Ramp | 1,032 | 1,096 | 1,781 | 1,114 | 168 | 213 | 188 | 147 | 864 | 883 | 1,593 | 967 |
| Mainline | 4,989 | 4,835 | 4,246 | 4,546 | 537 | 647 | 647 | 582 | 4,452 | 4,188 | 3,599 | 3,964 |
| Bollinger Loop On | 291 | 241 | 204 | 243 | 58 | 57 | 53 | 60 | 233 | 184 | 151 | 183 |
| Mainline | 5,280 | 5,076 | 4,450 | 4,789 | 595 | 704 | 700 | 642 | 4,685 | 4,372 | 3,750 | 4,147 |
| Bollinger Diagonal On | 861 | 980 | 819 | 749 | 166 | 189 | 124 | 112 | 695 | 791 | 695 | 637 |
| Mainline | 6,141 | 6,056 | 5,269 | 5,538 | 761 | 893 | 824 | 754 | 5,380 | 5,163 | 4,445 | 4,784 |
| Crow Canyon Off-Ramp | 1,317 | 1,094 | 1,771 | 1,240 | 109 | 132 | 125 | 86 | 1,208 | 962 | 1,646 | 1,154 |
| Mainline | 4,824 | 4,962 | 3,498 | 4,298 | 652 | 761 | 699 | 668 | 4,172 | 4,201 | 2,799 | 3,630 |
| Crow Canyon Loop On | 646 | 760 | 679 | 575 | 132 | 173 | 171 | 148 | 514 | 587 | 508 | 427 |
| Mainline | 5,470 | 5,722 | 4,177 | 4,873 | 784 | 934 | 870 | 816 | 4,686 | 4,788 | 3,307 | 4,057 |
| Crow Canyon Diagonal On | 963 | 1,059 | 1,007 | 723 | 193 | 241 | 270 | 180 | 770 | 818 | 737 | 543 |
| Mainline | 6,433 | 6,781 | 5,184 | 5,596 | 977 | 1,175 | 1,140 | 996 | 5,456 | 5,606 | 4,044 | 4,600 |
| Sycamore Off-Ramp | 581 | 795 | 489 | 678 | 163 | 169 | 150 | 168 | 418 | 626 | 339 | 510 |
| Mainline | 5,852 | 5,986 | 4,695 | 4,918 | 814 | 1,006 | 990 | 828 | 5,038 | 4,980 | 3,705 | 4,090 |
| Sycamore On | 1,121 | 1,052 | 1,021 | 948 | 257 | 247 | 227 | 205 | 864 | 805 | 794 | 743 |
| Mainline | 6,973 | 7,038 | 5,716 | 5,866 | 1,071 | 1,253 | 1,217 | 1,033 | 5,902 | 5,785 | 4,499 | 4,833 |
| Diablo Valley Off-Ramp | 724 | 728 | 647 | 679 | 79 | 78 | 51 | 41 | 645 | 650 | 596 | 638 |
| Mainline | 6,249 | 6,310 | 5,069 | 5,187 | 992 | 1,175 | 1,166 | 992 | 5,257 | 5,135 | 3,903 | 4,195 |
| Diablo Valley Loop On | 320 | 227 | 251 | 212 | 64 | 54 | 66 | 51 | 256 | 173 | 185 | 161 |
| Mainline | 6,569 | 6,537 | 5,320 | 5,399 | 1,056 | 1,229 | 1,232 | 1,043 | 5,513 | 5,308 | 4,088 | 4,356 |
| Diablo Valley Diagonal On | 166 | 210 | 202 | 164 | 34 | 50 | 52 | 41 | 132 | 160 | 150 | 123 |
| Mainline | 6,735 | 6,747 | 5,522 | 5,563 | 1,090 | 1,279 | 1,284 | 1,084 | 5,645 | 5,468 | 4,238 | 4,479 |
| El Cerro Off-Ramp | 426 | 487 | 441 | 441 | 42 | 50 | 31 | 24 | 384 | 437 | 410 | 417 |
| Mainline | 6,309 | 6,260 | 5,081 | 5,122 | 1,048 | 1,229 | 1,253 | 1,060 | 5,261 | 5,031 | 3,828 | 4,062 |
| El Cerro On | 664 | 409 | 450 | 372 | 114 | 121 | 119 | 91 | 550 | 288 | 331 | 281 |
| Mainline | 6,973 | 6,669 | 5,531 | 5,494 | 1,162 | 1,350 | 1,372 | 1,151 | 5,811 | 5,319 | 4,159 | 4,343 |
| El Pintado On | 125 | 96 | 85 | 59 | 26 | 24 | 23 | 15 | 99 | 72 | 62 | 44 |
| Mainline | 7,098 | 6,765 | 5,616 | 5,553 | 1,188 | 1,374 | 1,395 | 1,166 | 5,910 | 5,391 | 4,221 | 4,387 |
| Stone Valley Diagonal Off | 180 | 191 | 184 | 144 | 25 | 33 | 34 | 26 | 155 | 158 | 150 | 118 |
| Mainline | 6,918 | 6,574 | 5,432 | 5,409 | 1,163 | 1,341 | 1,361 | 1,140 | 5,755 | 5,233 | 4,071 | 4,269 |
| Stone Valley Loop Off | 300 | 228 | 211 | 185 | 41 | 39 | 35 | 28 | 259 | 189 | 176 | 157 |
| Mainline | 6,618 | 6,346 | 5,221 | 5,224 | 1,122 | 1,302 | 1,326 | 1,112 | 5,496 | 5,044 | 3,895 | 4,112 |
| Stone Valley On | 583 | 530 | 491 | 436 | 87 | 91 | 88 | 70 | 496 | 439 | 403 | 366 |
| Mainline | 7,201 | 6,876 | 5,712 | 5,660 | 1,209 | 1,393 | 1,414 | 1,182 | 5,992 | 5,483 | 4,298 | 4,478 |


|  | 2015 (Existing) Total Demand Volumes |  |  |  | 2015 (Existing) HOV Demand Volumes |  |  |  | 2015 (Existing) SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 244 | 204 | 235 | 223 | 34 | 34 | 40 | 37 | 210 | 170 | 195 | 186 |
| Mainline | 6,957 | 6,672 | 5,477 | 5,437 | 1,175 | 1,359 | 1,374 | 1,145 | 5,782 | 5,313 | 4,103 | 4,292 |
| Livorna On | 413 | 255 | 232 | 195 | 52 | 49 | 47 | 38 | 361 | 206 | 185 | 157 |
| Mainline | 7,370 | 6,927 | 5,709 | 5,632 | 1,227 | 1,408 | 1,421 | 1,183 | 6,143 | 5,519 | 4,288 | 4,449 |
| Rudgear Off | 552 | 477 | 481 | 466 | 63 | 84 | 99 | 81 | 489 | 393 | 382 | 385 |
| Mainline | 6,818 | 6,450 | 5,228 | 5,166 | 1,164 | 1,324 | 1,322 | 1,102 | 5,654 | 5,126 | 3,906 | 4,064 |
| Rudgear On | 435 | 474 | 516 | 430 | 66 | 82 | 95 | 72 | 369 | 392 | 421 | 358 |
| Mainline | 7,253 | 6,924 | 5,744 | 5,596 | 1,230 | 1,406 | 1,417 | 1,174 | 6,023 | 5,518 | 4,327 | 4,422 |
| S. Main Off | 460 | 341 | 285 | 301 | 62 | 61 | 67 | 62 | 398 | 280 | 218 | 239 |
| Mainline | 6,793 | 6,583 | 5,459 | 5,295 | 1,168 | 1,345 | 1,350 | 1,112 | 5,625 | 5,238 | 4,109 | 4,183 |
| Olympic Off | 559 | 920 | 811 | 753 | 60 | 128 | 147 | 92 | 499 | 792 | 664 | 661 |
| Mainline | 6,234 | 5,663 | 4,648 | 4,542 | 1,108 | 1,217 | 1,203 | 1,020 | 5,126 | 4,446 | 3,445 | 3,522 |
| SR 24 Off | 1,394 | 1,354 | 1,177 | 1,302 | 179 | 241 | 248 | 206 | 1,215 | 1,113 | 929 | 1,096 |
| Mainline | 4,840 | 4,309 | 3,471 | 3,240 | 929 | 976 | 955 | 814 | 3,911 | 3,333 | 2,516 | 2,426 |
| Olympic On | 1,290 | 1,081 | 1,446 | 1,024 | 179 | 224 | 242 | 162 | 1,111 | 857 | 1,204 | 862 |
| Mainline | 6,130 | 5,390 | 4,917 | 4,264 | 1,108 | 1,200 | 1,197 | 976 | 5,022 | 4,190 | 3,720 | 3,288 |
| Ygnacio Valley Off | 810 | 843 | 662 | 698 | 118 | 137 | 134 | 136 | 692 | 706 | 528 | 562 |
| Mainline | 5,320 | 4,547 | 4,255 | 3,566 | 990 | 1,063 | 1,063 | 840 | 4,330 | 3,484 | 3,192 | 2,726 |
| SR 24 On | 4,949 | 5,378 | 5,494 | 4,502 | 471 | 512 | 479 | 425 | 4,478 | 4,866 | 5,015 | 4,077 |
| Mainline | 10,269 | 9,925 | 9,749 | 8,068 | 1,461 | 1,575 | 1,542 | 1,265 | 8,808 | 8,350 | 8,207 | 6,803 |
| N. Main Off | 352 | 327 | 324 | 317 | 54 | 57 | 57 | 49 | 298 | 270 | 267 | 268 |
| Mainline | 9,917 | 9,598 | 9,425 | 7,751 | 1,407 | 1,518 | 1,485 | 1,216 | 8,510 | 8,080 | 7,940 | 6,535 |
| Lawrence On | 1,432 | 1,348 | 1,043 | 1,103 | 216 | 235 | 184 | 189 | 1,216 | 1,113 | 859 | 914 |
| Mainline | 11,349 | 10,946 | 10,468 | 8,854 | 1,623 | 1,753 | 1,669 | 1,405 | 9,726 | 9,193 | 8,799 | 7,449 |
| Treat Off | 1,641 | 1,391 | 1,423 | 1,545 | 256 | 244 | 246 | 237 | 1,385 | 1,147 | 1,177 | 1,308 |
| Mainline | 9,708 | 9,555 | 9,045 | 7,309 | 1,367 | 1,509 | 1,423 | 1,168 | 8,341 | 8,046 | 7,622 | 6,141 |
| Truck Scales On | 2 | 3 | 6 | 7 | 0 | 0 | 0 | 0 | 2 | 3 | 6 | 7 |
| Mainline | 9,710 | 9,558 | 9,051 | 7,316 | 1,367 | 1,509 | 1,423 | 1,168 | 8,343 | 8,049 | 7,628 | 6,148 |
| Buskirk On | 942 | 1,268 | 1,297 | 927 | 184 | 209 | 252 | 217 | 758 | 1,059 | 1,045 | 710 |
| Mainline | 10,652 | 10,826 | 10,348 | 8,243 | 1,551 | 1,718 | 1,675 | 1,385 | 9,101 | 9,108 | 8,673 | 6,858 |
| Oak On | 441 | 519 | 688 | 552 | 87 | 102 | 135 | 117 | 354 | 417 | 553 | 435 |
| Mainline | 11,093 | 11,345 | 11,036 | 8,795 | 1,638 | 1,820 | 1,810 | 1,502 | 9,455 | 9,525 | 9,226 | 7,293 |
| Contra Costa Off | 669 | 630 | 781 | 639 | 97 | 100 | 123 | 102 | 572 | 530 | 658 | 537 |
| Mainline | 10,424 | 10,715 | 10,255 | 8,156 | 1,541 | 1,720 | 1,687 | 1,400 | 8,883 | 8,995 | 8,568 | 6,756 |
| Monument Off | 935 | 899 | 786 | 614 | 143 | 147 | 134 | 111 | 792 | 752 | 652 | 503 |


|  | 2015 (Existing) Total Demand Volumes |  |  |  | 2015 (Existing) HOV Demand Volumes |  |  |  | 2015 (Existing) SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 9,489 | 9,816 | 9,469 | 7,542 | 1,398 | 1,573 | 1,553 | 1,289 | 8,091 | 8,243 | 7,916 | 6,253 |
| Monument On | 1,123 | 1,179 | 1,363 | 1,085 | 219 | 229 | 261 | 266 | 904 | 950 | 1,102 | 819 |
| Mainline | 10,612 | 10,995 | 10,832 | 8,627 | 1,617 | 1,802 | 1,814 | 1,555 | 8,995 | 9,193 | 9,018 | 7,072 |
| SR 242 Off | 5,316 | 5,357 | 5,289 | 4,282 | 795 | 875 | 876 | 755 | 4,521 | 4,482 | 4,413 | 3,527 |
| Mainline | 5,296 | 5,638 | 5,543 | 4,345 | 822 | 927 | 938 | 800 | 4,474 | 4,711 | 4,605 | 3,545 |
| Willow Pass Off | 1,087 | 1,022 | 923 | 878 | 177 | 172 | 162 | 172 | 910 | 850 | 761 | 706 |
| Mainline | 4,209 | 4,616 | 4,620 | 3,467 | 645 | 755 | 776 | 628 | 3,564 | 3,861 | 3,844 | 2,839 |
| Willow Pass On | 878 | 966 | 1,140 | 962 | 171 | 189 | 222 | 238 | 707 | 777 | 918 | 724 |
| Mainline | 5,087 | 5,582 | 5,760 | 4,429 | 816 | 944 | 998 | 866 | 4,271 | 4,638 | 4,762 | 3,563 |
| Burnett Off | 662 | 649 | 603 | 533 | 108 | 110 | 106 | 104 | 554 | 539 | 497 | 429 |
| Mainline | 4,425 | 4,933 | 5,157 | 3,896 | 708 | 834 | 892 | 762 | 3,717 | 4,099 | 4,265 | 3,134 |
| Burnett On | 558 | 584 | 624 | 516 | 100 | 104 | 111 | 92 | 458 | 480 | 513 | 424 |
| Mainline | 4,983 | 5,517 | 5,781 | 4,412 | 808 | 938 | 1,003 | 854 | 4,175 | 4,579 | 4,778 | 3,558 |
| Concord On | 338 | 377 | 430 | 332 | 114 | 101 | 88 | 90 | 224 | 276 | 342 | 242 |
| Mainline: NB I-680 north of Concord | 5,321 | 5,894 | 6,211 | 4,744 | 922 | 1,039 | 1,091 | 944 | 4,399 | 4,855 | 5,120 | 3,800 |

Note: Trucks are included in the SOV demand volume.

## APRIL/MAY 2015 OBSERVED INRIX SPEED DATA AND MODELED SPEED CONTOUR MAPS

## OBSERVED (2015) AND MODELED SPEED CONTOUR MAPS



YEAR 2020 TRAFFIC DEMAND FORECASTS

| ALTERNATIVE 1, 2, 5, 8 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 6,322 | 6,228 | 6,328 | 5,943 | 740 | 903 | 877 | 765 | 5,582 | 5,325 | 5,451 | 5,178 |
| Bollinger Off-Ramp | 1,084 | 1,151 | 1,870 | 1,170 | 176 | 224 | 197 | 154 | 908 | 927 | 1,673 | 1,016 |
| Mainline | 5,238 | 5,077 | 4,458 | 4,773 | 564 | 679 | 680 | 611 | 4,674 | 4,398 | 3,778 | 4,162 |
| Bollinger Loop On | 306 | 253 | 214 | 255 | 61 | 60 | 56 | 63 | 245 | 193 | 158 | 192 |
| Mainline | 5,544 | 5,330 | 4,672 | 5,028 | 625 | 739 | 736 | 674 | 4,919 | 4,591 | 3,936 | 4,354 |
| Bollinger Diagonal On | 904 | 1,029 | 860 | 786 | 174 | 198 | 130 | 118 | 730 | 831 | 730 | 668 |
| Mainline | 6,448 | 6,359 | 5,532 | 5,814 | 799 | 937 | 866 | 792 | 5,649 | 5,422 | 4,666 | 5,022 |
| Crow Canyon Off-Ramp | 1,383 | 1,149 | 1,860 | 1,302 | 114 | 139 | 131 | 90 | 1,269 | 1,010 | 1,729 | 1,212 |
| Mainline | 5,065 | 5,210 | 3,672 | 4,512 | 685 | 798 | 735 | 702 | 4,380 | 4,412 | 2,937 | 3,810 |
| Crow Canyon Loop On | 678 | 798 | 713 | 604 | 139 | 182 | 180 | 155 | 539 | 616 | 533 | 449 |
| Mainline | 5,743 | 6,008 | 4,385 | 5,116 | 824 | 980 | 915 | 857 | 4,919 | 5,028 | 3,470 | 4,259 |
| Crow Canyon Diagonal On | 1,011 | 1,112 | 1,057 | 759 | 203 | 253 | 284 | 189 | 808 | 859 | 773 | 570 |
| Mainline | 6,754 | 7,120 | 5,442 | 5,875 | 1,027 | 1,233 | 1,199 | 1,046 | 5,727 | 5,887 | 4,243 | 4,829 |
| Sycamore Off-Ramp | 610 | 835 | 513 | 712 | 171 | 177 | 158 | 176 | 439 | 658 | 355 | 536 |
| Mainline | 6,144 | 6,285 | 4,929 | 5,163 | 856 | 1,056 | 1,041 | 870 | 5,288 | 5,229 | 3,888 | 4,293 |
| Sycamore On | 1,177 | 1,105 | 1,072 | 995 | 270 | 259 | 238 | 215 | 907 | 846 | 834 | 780 |
| Mainline | 7,321 | 7,390 | 6,001 | 6,158 | 1,126 | 1,315 | 1,279 | 1,085 | 6,195 | 6,075 | 4,722 | 5,073 |
| Diablo Valley Off-Ramp | 760 | 764 | 679 | 713 | 83 | 82 | 54 | 43 | 677 | 682 | 625 | 670 |
| Mainline | 6,561 | 6,626 | 5,322 | 5,445 | 1,043 | 1,233 | 1,225 | 1,042 | 5,518 | 5,393 | 4,097 | 4,403 |
| Diablo Valley Loop On | 336 | 238 | 264 | 223 | 67 | 57 | 69 | 54 | 269 | 181 | 195 | 169 |
| Mainline | 6,897 | 6,864 | 5,586 | 5,668 | 1,110 | 1,290 | 1,294 | 1,096 | 5,787 | 5,574 | 4,292 | 4,572 |
| Diablo Valley Diagonal On | 174 | 221 | 212 | 172 | 36 | 53 | 55 | 43 | 138 | 168 | 157 | 129 |
| Mainline | 7,071 | 7,085 | 5,798 | 5,840 | 1,146 | 1,343 | 1,349 | 1,139 | 5,925 | 5,742 | 4,449 | 4,701 |
| El Cerro Off-Ramp | 447 | 511 | 463 | 463 | 44 | 53 | 33 | 25 | 403 | 458 | 430 | 438 |
| Mainline | 6,624 | 6,574 | 5,335 | 5,377 | 1,102 | 1,290 | 1,316 | 1,114 | 5,522 | 5,284 | 4,019 | 4,263 |
| El Cerro On | 697 | 429 | 473 | 391 | 120 | 127 | 125 | 96 | 577 | 302 | 348 | 295 |
| Mainline | 7,321 | 7,003 | 5,808 | 5,768 | 1,222 | 1,417 | 1,441 | 1,210 | 6,099 | 5,586 | 4,367 | 4,558 |
| El Pintado On | 131 | 101 | 89 | 62 | 27 | 25 | 24 | 16 | 104 | 76 | 65 | 46 |
| Mainline | 7,452 | 7,104 | 5,897 | 5,830 | 1,249 | 1,442 | 1,465 | 1,226 | 6,203 | 5,662 | 4,432 | 4,604 |
| Stone Valley Diagonal Off | 189 | 201 | 193 | 151 | 26 | 35 | 36 | 27 | 163 | 166 | 157 | 124 |
| Mainline | 7,263 | 6,903 | 5,704 | 5,679 | 1,223 | 1,407 | 1,429 | 1,199 | 6,040 | 5,496 | 4,275 | 4,480 |
| Stone Valley Loop Off | 315 | 239 | 222 | 194 | 43 | 41 | 37 | 29 | 272 | 198 | 185 | 165 |
| Mainline | 6,948 | 6,664 | 5,482 | 5,485 | 1,180 | 1,366 | 1,392 | 1,170 | 5,768 | 5,298 | 4,090 | 4,315 |
| Stone Valley On | 612 | 557 | 516 | 458 | 91 | 96 | 92 | 74 | 521 | 461 | 424 | 384 |
| Mainline | 7,560 | 7,221 | 5,998 | 5,943 | 1,271 | 1,462 | 1,484 | 1,244 | 6,289 | 5,759 | 4,514 | 4,699 |


| ALTERNATIVE 1, 2, 5, 8 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 256 | 214 | 247 | 234 | 36 | 36 | 42 | 39 | 220 | 178 | 205 | 195 |
| Mainline | 7,304 | 7,007 | 5,751 | 5,709 | 1,235 | 1,426 | 1,442 | 1,205 | 6,069 | 5,581 | 4,309 | 4,504 |
| Livorna On | 434 | 268 | 244 | 205 | 55 | 51 | 49 | 40 | 379 | 217 | 195 | 165 |
| Mainline | 7,738 | 7,275 | 5,995 | 5,914 | 1,290 | 1,477 | 1,491 | 1,245 | 6,448 | 5,798 | 4,504 | 4,669 |
| Rudgear Off | 580 | 501 | 505 | 489 | 66 | 88 | 104 | 85 | 514 | 413 | 401 | 404 |
| Mainline | 7,158 | 6,774 | 5,490 | 5,425 | 1,224 | 1,389 | 1,387 | 1,160 | 5,934 | 5,385 | 4,103 | 4,265 |
| Rudgear On | 457 | 498 | 542 | 452 | 69 | 86 | 100 | 76 | 388 | 412 | 442 | 376 |
| Mainline | 7,615 | 7,272 | 6,032 | 5,877 | 1,293 | 1,475 | 1,487 | 1,236 | 6,322 | 5,797 | 4,545 | 4,641 |
| S. Main Off | 483 | 358 | 299 | 316 | 65 | 64 | 70 | 65 | 418 | 294 | 229 | 251 |
| Mainline | 7,132 | 6,914 | 5,733 | 5,561 | 1,228 | 1,411 | 1,417 | 1,171 | 5,904 | 5,503 | 4,316 | 4,390 |
| Olympic Off | 587 | 966 | 852 | 791 | 63 | 134 | 154 | 97 | 524 | 832 | 698 | 694 |
| Mainline | 6,545 | 5,948 | 4,881 | 4,770 | 1,165 | 1,277 | 1,263 | 1,074 | 5,380 | 4,671 | 3,618 | 3,696 |
| SR 24 Off | 1,464 | 1,422 | 1,236 | 1,367 | 188 | 253 | 260 | 216 | 1,276 | 1,169 | 976 | 1,151 |
| Mainline | 5,081 | 4,526 | 3,645 | 3,403 | 977 | 1,024 | 1,003 | 858 | 4,104 | 3,502 | 2,642 | 2,545 |
| Olympic On | 1,355 | 1,135 | 1,518 | 1,075 | 188 | 235 | 254 | 170 | 1,167 | 900 | 1,264 | 905 |
| Mainline | 6,436 | 5,661 | 5,163 | 4,478 | 1,165 | 1,259 | 1,257 | 1,028 | 5,271 | 4,402 | 3,906 | 3,450 |
| Ygnacio Valley Off | 851 | 885 | 695 | 733 | 124 | 144 | 141 | 143 | 727 | 741 | 554 | 590 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| SR 24 On | 5,196 | 5,647 | 5,769 | 4,727 | 495 | 538 | 503 | 446 | 4,701 | 5,109 | 5,266 | 4,281 |
| Mainline | 10,781 | 10,423 | 10,237 | 8,472 | 1,536 | 1,653 | 1,619 | 1,331 | 9,245 | 8,770 | 8,618 | 7,141 |
| N. Main Off | 370 | 343 | 340 | 333 | 57 | 60 | 60 | 51 | 313 | 283 | 280 | 282 |
| Mainline | 10,411 | 10,080 | 9,897 | 8,139 | 1,479 | 1,593 | 1,559 | 1,280 | 8,932 | 8,487 | 8,338 | 6,859 |
| Lawrence On | 1,504 | 1,415 | 1,095 | 1,158 | 227 | 247 | 193 | 198 | 1,277 | 1,168 | 902 | 960 |
| Mainline | 11,915 | 11,495 | 10,992 | 9,297 | 1,706 | 1,840 | 1,752 | 1,478 | 10,209 | 9,655 | 9,240 | 7,819 |
| Treat Off | 1,723 | 1,461 | 1,494 | 1,622 | 269 | 256 | 258 | 249 | 1,454 | 1,205 | 1,236 | 1,373 |
| Mainline | 10,192 | 10,034 | 9,498 | 7,675 | 1,437 | 1,584 | 1,494 | 1,229 | 8,755 | 8,450 | 8,004 | 6,446 |
| Truck Scales On | 2 | 5 | 6 | 7 | 0 | 0 | 0 | 0 | 2 | 5 | 6 | 7 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Buskirk On | 989 | 1,331 | 1,362 | 973 | 193 | 219 | 265 | 228 | 796 | 1,112 | 1,097 | 745 |
| Mainline | 11,183 | 11,370 | 10,866 | 8,655 | 1,630 | 1,803 | 1,759 | 1,457 | 9,553 | 9,567 | 9,107 | 7,198 |
| Oak On | 463 | 545 | 722 | 580 | 91 | 107 | 142 | 123 | 372 | 438 | 580 | 457 |
| Mainline | 11,646 | 11,915 | 11,588 | 9,235 | 1,721 | 1,910 | 1,901 | 1,580 | 9,925 | 10,005 | 9,687 | 7,655 |
| Contra Costa Off | 702 | 662 | 820 | 671 | 102 | 105 | 129 | 107 | 600 | 557 | 691 | 564 |
| Mainline | 10,944 | 11,253 | 10,768 | 8,564 | 1,619 | 1,805 | 1,772 | 1,473 | 9,325 | 9,448 | 8,996 | 7,091 |
| Monument Off | 982 | 944 | 825 | 645 | 150 | 154 | 141 | 117 | 832 | 790 | 684 | 528 |


| ALTERNATIVE 1, 2, 5, 8 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 9,962 | 10,309 | 9,943 | 7,919 | 1,469 | 1,651 | 1,631 | 1,356 | 8,493 | 8,658 | 8,312 | 6,563 |
| Monument On | 1,179 | 1,238 | 1,431 | 1,139 | 230 | 240 | 274 | 279 | 949 | 998 | 1,157 | 860 |
| Mainline | 11,141 | 11,547 | 11,374 | 9,058 | 1,699 | 1,891 | 1,905 | 1,635 | 9,442 | 9,656 | 9,469 | 7,423 |
| SR 242 Off | 5,582 | 5,625 | 5,553 | 4,496 | 835 | 919 | 920 | 793 | 4,747 | 4,706 | 4,633 | 3,703 |
| Mainline | 5,559 | 5,922 | 5,821 | 4,562 | 864 | 972 | 985 | 842 | 4,695 | 4,950 | 4,836 | 3,720 |
| Willow Pass Off | 1,141 | 1,073 | 969 | 922 | 186 | 181 | 170 | 181 | 955 | 892 | 799 | 741 |
| Mainline | 4,418 | 4,849 | 4,852 | 3,640 | 678 | 791 | 815 | 661 | 3,740 | 4,058 | 4,037 | 2,979 |
| Willow Pass On | 922 | 1,014 | 1,197 | 1,010 | 180 | 198 | 233 | 250 | 742 | 816 | 964 | 760 |
| Mainline | 5,340 | 5,863 | 6,049 | 4,650 | 858 | 989 | 1,048 | 911 | 4,482 | 4,874 | 5,001 | 3,739 |
| Burnett Off | 695 | 681 | 633 | 560 | 113 | 116 | 111 | 109 | 582 | 565 | 522 | 451 |
| Mainline | 4,645 | 5,182 | 5,416 | 4,090 | 745 | 873 | 937 | 802 | 3,900 | 4,309 | 4,479 | 3,288 |
| Burnett On | 586 | 613 | 655 | 542 | 105 | 109 | 117 | 97 | 481 | 504 | 538 | 445 |
| Mainline | 5,231 | 5,795 | 6,071 | 4,632 | 850 | 982 | 1,054 | 899 | 4,381 | 4,813 | 5,017 | 3,733 |
| Concord On | 355 | 396 | 452 | 349 | 120 | 106 | 92 | 95 | 235 | 290 | 360 | 254 |
| Mainline: NB I-680 north of Concord | 5,586 | 6,191 | 6,523 | 4,981 | 970 | 1,088 | 1,146 | 994 | 4,616 | 5,103 | 5,377 | 3,987 |

Note: Trucks are included in the SOV demand volume.

| ALTERNATIVE 3 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 4,962 | 4,877 | 4,954 | 4,663 | 678 | 823 | 803 | 696 | 4,283 | 4,052 | 4,153 | 3,967 |
| Bollinger Off-Ramp | 867 | 921 | 1,496 | 936 | 141 | 179 | 158 | 123 | 726 | 742 | 1,338 | 813 |
| Mainline | 4,095 | 3,956 | 3,458 | 3,727 | 537 | 644 | 645 | 573 | 3,557 | 3,310 | 2,815 | 3,154 |
| Bollinger Loop On | 241 | 200 | 170 | 199 | 50 | 49 | 46 | 51 | 191 | 151 | 124 | 148 |
| Mainline | 4,336 | 4,156 | 3,628 | 3,926 | 587 | 693 | 691 | 624 | 3,748 | 3,461 | 2,939 | 3,302 |
| Bollinger Diagonal On | 711 | 811 | 676 | 620 | 141 | 160 | 106 | 96 | 570 | 651 | 570 | 524 |
| Mainline | 5,047 | 4,967 | 4,304 | 4,546 | 728 | 853 | 797 | 720 | 4,318 | 4,112 | 3,509 | 3,826 |
| Crow Canyon Off-Ramp | 1,106 | 919 | 1,488 | 1,042 | 91 | 111 | 105 | 72 | 1,015 | 808 | 1,383 | 970 |
| Mainline | 3,941 | 4,048 | 2,816 | 3,504 | 637 | 742 | 692 | 648 | 3,303 | 3,304 | 2,126 | 2,856 |
| Crow Canyon Loop On | 535 | 625 | 558 | 474 | 113 | 147 | 146 | 126 | 422 | 478 | 412 | 348 |
| Mainline | 4,476 | 4,673 | 3,374 | 3,978 | 750 | 889 | 838 | 774 | 3,725 | 3,782 | 2,538 | 3,204 |
| Crow Canyon Diagonal On | 788 | 872 | 827 | 586 | 164 | 204 | 229 | 153 | 624 | 668 | 598 | 434 |
| Mainline | 5,264 | 5,545 | 4,201 | 4,564 | 914 | 1,093 | 1,067 | 927 | 4,349 | 4,450 | 3,136 | 3,638 |
| Sycamore Off-Ramp | 488 | 668 | 410 | 570 | 137 | 142 | 126 | 141 | 351 | 526 | 284 | 429 |
| Mainline | 4,776 | 4,877 | 3,791 | 3,994 | 777 | 951 | 941 | 786 | 3,998 | 3,924 | 2,852 | 3,209 |
| Sycamore On | 942 | 884 | 858 | 796 | 216 | 207 | 190 | 172 | 726 | 677 | 667 | 624 |
| Mainline | 5,718 | 5,761 | 4,649 | 4,790 | 993 | 1,158 | 1,131 | 958 | 4,724 | 4,601 | 3,519 | 3,833 |
| Diablo Valley Off-Ramp | 608 | 611 | 543 | 570 | 66 | 66 | 43 | 34 | 542 | 546 | 500 | 536 |
| Mainline | 5,110 | 5,150 | 4,106 | 4,220 | 927 | 1,092 | 1,088 | 924 | 4,182 | 4,055 | 3,019 | 3,297 |
| Diablo Valley Loop On | 269 | 190 | 211 | 178 | 54 | 46 | 55 | 43 | 215 | 145 | 156 | 135 |
| Mainline | 5,379 | 5,340 | 4,317 | 4,398 | 981 | 1,138 | 1,143 | 967 | 4,397 | 4,200 | 3,175 | 3,432 |
| Diablo Valley Diagonal On | 139 | 177 | 170 | 138 | 29 | 42 | 44 | 34 | 110 | 134 | 126 | 103 |
| Mainline | 5,518 | 5,517 | 4,487 | 4,536 | 1,010 | 1,180 | 1,187 | 1,001 | 4,507 | 4,334 | 3,301 | 3,535 |
| El Cerro Off-Ramp | 358 | 409 | 370 | 370 | 35 | 42 | 26 | 20 | 322 | 366 | 344 | 350 |
| Mainline | 5,160 | 5,108 | 4,117 | 4,166 | 975 | 1,138 | 1,161 | 981 | 4,185 | 3,968 | 2,957 | 3,185 |
| El Cerro On | 558 | 343 | 378 | 313 | 96 | 102 | 100 | 77 | 462 | 242 | 278 | 236 |
| Mainline | 5,718 | 5,451 | 4,495 | 4,479 | 1,071 | 1,240 | 1,261 | 1,058 | 4,647 | 4,210 | 3,235 | 3,421 |
| El Pintado On | 105 | 81 | 71 | 50 | 22 | 20 | 19 | 13 | 83 | 61 | 52 | 37 |
| Mainline | 5,823 | 5,532 | 4,566 | 4,529 | 1,093 | 1,260 | 1,280 | 1,071 | 4,730 | 4,271 | 3,287 | 3,458 |
| Stone Valley Diagonal Off | 151 | 161 | 154 | 121 | 21 | 28 | 29 | 22 | 130 | 133 | 126 | 99 |
| Mainline | 5,672 | 5,371 | 4,412 | 4,408 | 1,072 | 1,232 | 1,251 | 1,049 | 4,600 | 4,138 | 3,161 | 3,359 |
| Stone Valley Loop Off | 252 | 191 | 178 | 155 | 34 | 33 | 30 | 23 | 218 | 158 | 148 | 132 |
| Mainline | 5,420 | 5,180 | 4,234 | 4,253 | 1,038 | 1,199 | 1,221 | 1,026 | 4,382 | 3,980 | 3,013 | 3,227 |
| Stone Valley On | 490 | 446 | 413 | 366 | 73 | 77 | 74 | 59 | 417 | 369 | 339 | 307 |
| Mainline | 5,910 | 5,626 | 4,647 | 4,619 | 1,111 | 1,276 | 1,295 | 1,085 | 4,799 | 4,349 | 3,352 | 3,534 |


| ALTERNATIVE 3 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 205 | 171 | 198 | 187 | 29 | 29 | 34 | 31 | 176 | 142 | 164 | 156 |
| Mainline | 5,705 | 5,455 | 4,449 | 4,432 | 1,082 | 1,247 | 1,261 | 1,054 | 4,623 | 4,207 | 3,188 | 3,378 |
| Livorna On | 347 | 214 | 195 | 164 | 44 | 41 | 39 | 32 | 303 | 174 | 156 | 132 |
| Mainline | 6,052 | 5,669 | 4,644 | 4,596 | 1,126 | 1,288 | 1,300 | 1,086 | 4,926 | 4,381 | 3,344 | 3,510 |
| Rudgear Off | 464 | 401 | 404 | 391 | 53 | 70 | 83 | 68 | 411 | 330 | 321 | 323 |
| Mainline | 5,588 | 5,268 | 4,240 | 4,205 | 1,073 | 1,218 | 1,217 | 1,018 | 4,515 | 4,051 | 3,023 | 3,187 |
| Rudgear On | 366 | 398 | 434 | 362 | 55 | 69 | 80 | 61 | 310 | 330 | 354 | 301 |
| Mainline | 5,954 | 5,666 | 4,674 | 4,567 | 1,128 | 1,287 | 1,297 | 1,079 | 4,825 | 4,381 | 3,377 | 3,488 |
| S. Main Off | 386 | 286 | 239 | 253 | 52 | 51 | 56 | 52 | 334 | 235 | 183 | 201 |
| Mainline | 5,568 | 5,380 | 4,435 | 4,314 | 1,076 | 1,236 | 1,241 | 1,027 | 4,491 | 4,146 | 3,194 | 3,287 |
| Olympic Off | 470 | 773 | 682 | 633 | 50 | 107 | 123 | 78 | 419 | 666 | 558 | 555 |
| Mainline | 5,098 | 4,607 | 3,753 | 3,681 | 1,026 | 1,129 | 1,118 | 949 | 4,072 | 3,480 | 2,636 | 2,732 |
| SR 24 Off | 1,171 | 1,138 | 989 | 1,094 | 150 | 202 | 208 | 173 | 1,021 | 935 | 781 | 921 |
| Mainline | 3,927 | 3,469 | 2,764 | 2,587 | 876 | 927 | 910 | 776 | 3,051 | 2,545 | 1,855 | 1,811 |
| Olympic On | 1,084 | 908 | 1,214 | 860 | 150 | 188 | 203 | 136 | 934 | 720 | 1,011 | 724 |
| Mainline | 5,011 | 4,377 | 3,978 | 3,447 | 1,026 | 1,115 | 1,113 | 912 | 3,985 | 3,265 | 2,866 | 2,535 |
| Ygnacio Valley Off | 637 | 662 | 511 | 542 | 105 | 121 | 118 | 120 | 532 | 542 | 393 | 422 |
| Mainline | 4,374 | 3,715 | 3,467 | 2,905 | 921 | 994 | 995 | 792 | 3,453 | 2,723 | 2,473 | 2,113 |
| SR 24 On | 4,157 | 4,518 | 4,615 | 3,782 | 396 | 430 | 402 | 357 | 3,761 | 4,087 | 4,213 | 3,425 |
| Mainline | 8,531 | 8,233 | 8,082 | 6,687 | 1,317 | 1,424 | 1,397 | 1,149 | 7,214 | 6,810 | 6,686 | 5,538 |
| N. Main Off | 296 | 274 | 272 | 266 | 46 | 48 | 48 | 41 | 250 | 226 | 224 | 226 |
| Mainline | 8,235 | 7,959 | 7,810 | 6,421 | 1,271 | 1,376 | 1,349 | 1,108 | 6,964 | 6,584 | 6,462 | 5,312 |
| Lawrence On | 1,203 | 1,132 | 876 | 926 | 182 | 198 | 154 | 158 | 1,022 | 934 | 722 | 768 |
| Mainline | 9,438 | 9,091 | 8,686 | 7,347 | 1,453 | 1,574 | 1,503 | 1,266 | 7,986 | 7,518 | 7,184 | 6,080 |
| Treat Off | 1,378 | 1,169 | 1,195 | 1,298 | 215 | 205 | 206 | 199 | 1,163 | 964 | 989 | 1,098 |
| Mainline | 8,060 | 7,922 | 7,491 | 6,049 | 1,238 | 1,369 | 1,297 | 1,067 | 6,823 | 6,554 | 6,195 | 4,982 |
| Truck Scales On | 2 | 4 | 5 | 6 | 0 | 0 | 0 | 0 | 2 | 4 | 5 | 6 |
| Mainline | 8,062 | 7,926 | 7,496 | 6,055 | 1,238 | 1,369 | 1,297 | 1,067 | 6,825 | 6,558 | 6,200 | 4,988 |
| Buskirk On | 791 | 1,065 | 1,090 | 778 | 154 | 175 | 212 | 182 | 637 | 890 | 878 | 596 |
| Mainline | 8,853 | 8,991 | 8,586 | 6,833 | 1,392 | 1,544 | 1,509 | 1,249 | 7,462 | 7,448 | 7,078 | 5,584 |
| Oak On | 370 | 436 | 578 | 464 | 73 | 86 | 114 | 98 | 298 | 350 | 464 | 366 |
| Mainline | 9,223 | 9,427 | 9,164 | 7,297 | 1,465 | 1,630 | 1,623 | 1,347 | 7,760 | 7,798 | 7,542 | 5,950 |
| Contra Costa Off | 562 | 530 | 656 | 537 | 82 | 84 | 103 | 86 | 480 | 446 | 553 | 451 |
| Mainline | 8,661 | 8,897 | 8,508 | 6,760 | 1,383 | 1,546 | 1,520 | 1,261 | 7,280 | 7,352 | 6,989 | 5,499 |
| Monument Off | 786 | 755 | 660 | 516 | 120 | 123 | 113 | 94 | 666 | 632 | 547 | 422 |


| ALTERNATIVE 3 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 7,875 | 8,142 | 7,848 | 6,244 | 1,263 | 1,423 | 1,407 | 1,167 | 6,614 | 6,720 | 6,442 | 5,077 |
| Monument On | 943 | 990 | 1,145 | 911 | 184 | 192 | 219 | 223 | 759 | 798 | 926 | 688 |
| Mainline | 8,818 | 9,132 | 8,993 | 7,155 | 1,447 | 1,615 | 1,626 | 1,390 | 7,373 | 7,518 | 7,368 | 5,765 |
| SR 242 Off | 4,466 | 4,500 | 4,442 | 3,597 | 668 | 735 | 736 | 634 | 3,798 | 3,765 | 3,706 | 2,962 |
| Mainline | 4,352 | 4,632 | 4,551 | 3,558 | 779 | 880 | 890 | 756 | 3,575 | 3,753 | 3,662 | 2,803 |
| Willow Pass Off | 913 | 858 | 775 | 738 | 149 | 145 | 136 | 145 | 764 | 714 | 639 | 593 |
| Mainline | 3,439 | 3,774 | 3,776 | 2,820 | 630 | 735 | 754 | 611 | 2,811 | 3,039 | 3,023 | 2,210 |
| Willow Pass On | 738 | 811 | 958 | 808 | 144 | 158 | 186 | 200 | 594 | 653 | 771 | 608 |
| Mainline | 4,177 | 4,585 | 4,734 | 3,628 | 774 | 893 | 940 | 811 | 3,405 | 3,692 | 3,794 | 2,818 |
| Burnett Off | 556 | 545 | 506 | 448 | 90 | 93 | 89 | 87 | 466 | 452 | 418 | 361 |
| Mainline | 3,621 | 4,040 | 4,228 | 3,180 | 684 | 800 | 851 | 724 | 2,939 | 3,240 | 3,376 | 2,457 |
| Burnett On | 469 | 490 | 524 | 434 | 84 | 87 | 94 | 78 | 385 | 403 | 430 | 356 |
| Mainline | 4,090 | 4,530 | 4,752 | 3,614 | 768 | 887 | 945 | 802 | 3,324 | 3,643 | 3,806 | 2,813 |
| Concord On | 284 | 317 | 362 | 279 | 96 | 85 | 74 | 76 | 188 | 232 | 288 | 203 |
| Mainline: NB I-680 north of Concord | 4,374 | 4,847 | 5,114 | 3,893 | 864 | 972 | 1,019 | 878 | 3,512 | 3,875 | 4,094 | 3,016 |

Note: Trucks are included in the SOV demand volume.

| ALTERNATIVE 4 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 5,006 | 4,927 | 5,006 | 4,705 | 634 | 773 | 753 | 654 | 4,371 | 4,153 | 4,255 | 4,051 |
| Bollinger Off-Ramp | 867 | 921 | 1,496 | 936 | 141 | 179 | 158 | 123 | 726 | 742 | 1,338 | 813 |
| Mainline | 4,139 | 4,006 | 3,510 | 3,769 | 493 | 594 | 595 | 531 | 3,645 | 3,411 | 2,917 | 3,238 |
| Bollinger Loop On | 241 | 200 | 170 | 199 | 50 | 49 | 46 | 51 | 191 | 151 | 124 | 148 |
| Mainline | 4,380 | 4,206 | 3,680 | 3,968 | 543 | 643 | 641 | 582 | 3,836 | 3,562 | 3,041 | 3,386 |
| Bollinger Diagonal On | 711 | 811 | 676 | 620 | 141 | 160 | 106 | 96 | 570 | 651 | 570 | 524 |
| Mainline | 5,091 | 5,017 | 4,356 | 4,588 | 684 | 803 | 747 | 678 | 4,406 | 4,213 | 3,611 | 3,910 |
| Crow Canyon Off-Ramp | 1,106 | 919 | 1,488 | 1,042 | 91 | 111 | 105 | 72 | 1,015 | 808 | 1,383 | 970 |
| Mainline | 3,985 | 4,098 | 2,868 | 3,546 | 593 | 692 | 642 | 606 | 3,391 | 3,405 | 2,228 | 2,940 |
| Crow Canyon Loop On | 535 | 625 | 558 | 474 | 113 | 147 | 146 | 126 | 422 | 478 | 412 | 348 |
| Mainline | 4,520 | 4,723 | 3,426 | 4,020 | 706 | 839 | 788 | 732 | 3,813 | 3,883 | 2,640 | 3,288 |
| Crow Canyon Diagonal On | 788 | 872 | 827 | 586 | 164 | 204 | 229 | 153 | 624 | 668 | 598 | 434 |
| Mainline | 5,308 | 5,595 | 4,253 | 4,606 | 870 | 1,043 | 1,017 | 885 | 4,437 | 4,551 | 3,238 | 3,722 |
| Sycamore Off-Ramp | 488 | 668 | 410 | 570 | 137 | 142 | 126 | 141 | 351 | 526 | 284 | 429 |
| Mainline | 4,820 | 4,927 | 3,843 | 4,036 | 733 | 901 | 891 | 744 | 4,086 | 4,025 | 2,954 | 3,293 |
| Sycamore On | 942 | 884 | 858 | 796 | 216 | 207 | 190 | 172 | 726 | 677 | 667 | 624 |
| Mainline | 5,762 | 5,811 | 4,701 | 4,832 | 949 | 1,108 | 1,081 | 916 | 4,812 | 4,702 | 3,621 | 3,917 |
| Diablo Valley Off-Ramp | 608 | 611 | 543 | 570 | 66 | 66 | 43 | 34 | 542 | 546 | 500 | 536 |
| Mainline | 5,154 | 5,200 | 4,158 | 4,262 | 883 | 1,042 | 1,038 | 882 | 4,270 | 4,156 | 3,121 | 3,381 |
| Diablo Valley Loop On | 269 | 190 | 211 | 178 | 54 | 46 | 55 | 43 | 215 | 145 | 156 | 135 |
| Mainline | 5,423 | 5,390 | 4,369 | 4,440 | 937 | 1,088 | 1,093 | 925 | 4,485 | 4,301 | 3,277 | 3,516 |
| Diablo Valley Diagonal On | 139 | 177 | 170 | 138 | 29 | 42 | 44 | 34 | 110 | 134 | 126 | 103 |
| Mainline | 5,562 | 5,567 | 4,539 | 4,578 | 966 | 1,130 | 1,137 | 959 | 4,595 | 4,435 | 3,403 | 3,619 |
| El Cerro Off-Ramp | 358 | 409 | 370 | 370 | 35 | 42 | 26 | 20 | 322 | 366 | 344 | 350 |
| Mainline | 5,204 | 5,158 | 4,169 | 4,208 | 931 | 1,088 | 1,111 | 939 | 4,273 | 4,069 | 3,059 | 3,269 |
| El Cerro On | 558 | 343 | 378 | 313 | 96 | 102 | 100 | 77 | 462 | 242 | 278 | 236 |
| Mainline | 5,762 | 5,501 | 4,547 | 4,521 | 1,027 | 1,190 | 1,211 | 1,016 | 4,735 | 4,311 | 3,337 | 3,505 |
| El Pintado On | 105 | 81 | 71 | 50 | 22 | 20 | 19 | 13 | 83 | 61 | 52 | 37 |
| Mainline | 5,867 | 5,582 | 4,618 | 4,571 | 1,049 | 1,210 | 1,230 | 1,029 | 4,818 | 4,372 | 3,389 | 3,542 |
| Stone Valley Diagonal Off | 151 | 161 | 154 | 121 | 21 | 28 | 29 | 22 | 130 | 133 | 126 | 99 |
| Mainline | 5,716 | 5,421 | 4,464 | 4,450 | 1,028 | 1,182 | 1,201 | 1,007 | 4,688 | 4,239 | 3,263 | 3,443 |
| Stone Valley Loop Off | 252 | 191 | 178 | 155 | 34 | 33 | 30 | 23 | 218 | 158 | 148 | 132 |
| Mainline | 5,464 | 5,230 | 4,286 | 4,295 | 994 | 1,149 | 1,171 | 984 | 4,470 | 4,081 | 3,115 | 3,311 |
| Stone Valley On | 490 | 446 | 413 | 366 | 73 | 77 | 74 | 59 | 417 | 369 | 339 | 307 |
| Mainline | 5,954 | 5,676 | 4,699 | 4,661 | 1,067 | 1,226 | 1,245 | 1,043 | 4,887 | 4,450 | 3,454 | 3,618 |


| ALTERNATIVE 4 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 205 | 171 | 198 | 187 | 29 | 29 | 34 | 31 | 176 | 142 | 164 | 156 |
| Mainline | 5,749 | 5,505 | 4,501 | 4,474 | 1,038 | 1,197 | 1,211 | 1,012 | 4,711 | 4,308 | 3,290 | 3,462 |
| Livorna On | 347 | 214 | 195 | 164 | 44 | 41 | 39 | 32 | 303 | 174 | 156 | 132 |
| Mainline | 6,096 | 5,719 | 4,696 | 4,638 | 1,082 | 1,238 | 1,250 | 1,044 | 5,014 | 4,482 | 3,446 | 3,594 |
| Rudgear Off | 464 | 401 | 404 | 391 | 53 | 70 | 83 | 68 | 411 | 330 | 321 | 323 |
| Mainline | 5,632 | 5,318 | 4,292 | 4,247 | 1,029 | 1,168 | 1,167 | 976 | 4,603 | 4,152 | 3,125 | 3,271 |
| Rudgear On | 366 | 398 | 434 | 362 | 55 | 69 | 80 | 61 | 310 | 330 | 354 | 301 |
| Mainline | 5,998 | 5,716 | 4,726 | 4,609 | 1,084 | 1,237 | 1,247 | 1,037 | 4,913 | 4,482 | 3,479 | 3,572 |
| S. Main Off | 386 | 286 | 239 | 253 | 52 | 51 | 56 | 52 | 334 | 235 | 183 | 201 |
| Mainline | 5,612 | 5,430 | 4,487 | 4,356 | 1,032 | 1,186 | 1,191 | 985 | 4,579 | 4,247 | 3,296 | 3,371 |
| Olympic Off | 470 | 773 | 682 | 633 | 50 | 107 | 123 | 78 | 419 | 666 | 558 | 555 |
| Mainline | 5,142 | 4,657 | 3,805 | 3,723 | 982 | 1,079 | 1,068 | 907 | 4,160 | 3,581 | 2,738 | 2,816 |
| SR 24 Off | 1,171 | 1,138 | 989 | 1,094 | 150 | 202 | 208 | 173 | 1,021 | 935 | 781 | 921 |
| Mainline | 3,971 | 3,519 | 2,816 | 2,629 | 832 | 877 | 860 | 734 | 3,139 | 2,646 | 1,957 | 1,895 |
| Olympic On | 1,084 | 908 | 1,214 | 860 | 150 | 188 | 203 | 136 | 934 | 720 | 1,011 | 724 |
| Mainline | 5,055 | 4,427 | 4,030 | 3,489 | 982 | 1,065 | 1,063 | 870 | 4,073 | 3,366 | 2,968 | 2,619 |
| Ygnacio Valley Off | 637 | 662 | 511 | 542 | 105 | 121 | 118 | 120 | 532 | 542 | 393 | 422 |
| Mainline | 4,418 | 3,765 | 3,519 | 2,947 | 877 | 944 | 945 | 750 | 3,541 | 2,824 | 2,575 | 2,197 |
| SR 24 On | 4,157 | 4,518 | 4,615 | 3,782 | 396 | 430 | 402 | 357 | 3,761 | 4,087 | 4,213 | 3,425 |
| Mainline | 8,575 | 8,283 | 8,134 | 6,729 | 1,273 | 1,374 | 1,347 | 1,107 | 7,302 | 6,911 | 6,788 | 5,622 |
| N. Main Off/Treat Off | 1,673 | 1,439 | 1,462 | 1,558 | 261 | 253 | 254 | 240 | 1,412 | 1,186 | 1,208 | 1,318 |
| Mainline | 6,902 | 6,844 | 6,672 | 5,171 | 1,012 | 1,121 | 1,093 | 867 | 5,890 | 5,725 | 5,580 | 4,304 |
| Lawrence On | 1,203 | 1,132 | 876 | 926 | 182 | 198 | 154 | 158 | 1,022 | 934 | 722 | 768 |
| Mainline | 8,105 | 7,976 | 7,548 | 6,097 | 1,194 | 1,319 | 1,247 | 1,025 | 6,912 | 6,659 | 6,302 | 5,072 |
| Treat Off (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 8,105 | 7,976 | 7,548 | 6,097 | 1,194 | 1,319 | 1,247 | 1,025 | 6,912 | 6,659 | 6,302 | 5,072 |
| Truck Scales On (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 8,105 | 7,976 | 7,548 | 6,097 | 1,194 | 1,319 | 1,247 | 1,025 | 6,912 | 6,659 | 6,302 | 5,072 |
| Buskirk On | 791 | 1,065 | 1,090 | 778 | 154 | 175 | 212 | 182 | 637 | 890 | 878 | 596 |
| Mainline | 8,896 | 9,041 | 8,638 | 6,875 | 1,348 | 1,494 | 1,459 | 1,207 | 7,549 | 7,549 | 7,180 | 5,668 |
| Oak On | 370 | 436 | 578 | 464 | 73 | 86 | 114 | 98 | 298 | 350 | 464 | 366 |
| Mainline | 9,266 | 9,477 | 9,216 | 7,339 | 1,421 | 1,580 | 1,573 | 1,305 | 7,847 | 7,899 | 7,644 | 6,034 |
| Contra Costa Off | 562 | 530 | 656 | 537 | 82 | 84 | 103 | 86 | 480 | 446 | 553 | 451 |
| Mainline | 8,704 | 8,947 | 8,560 | 6,802 | 1,339 | 1,496 | 1,470 | 1,219 | 7,367 | 7,453 | 7,091 | 5,583 |
| Monument Off | 786 | 755 | 660 | 516 | 120 | 123 | 113 | 94 | 666 | 632 | 547 | 422 |


| ALTERNATIVE 4 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 7,918 | 8,192 | 7,900 | 6,286 | 1,219 | 1,373 | 1,357 | 1,125 | 6,701 | 6,821 | 6,544 | 5,161 |
| Monument On | 943 | 990 | 1,145 | 911 | 184 | 192 | 219 | 223 | 759 | 798 | 926 | 688 |
| Mainline | 8,861 | 9,182 | 9,045 | 7,197 | 1,403 | 1,565 | 1,576 | 1,348 | 7,460 | 7,619 | 7,470 | 5,849 |
| SR 242 Off | 4,466 | 4,500 | 4,442 | 3,597 | 668 | 735 | 736 | 634 | 3,798 | 3,765 | 3,706 | 2,962 |
| Mainline | 4,395 | 4,682 | 4,603 | 3,600 | 735 | 830 | 840 | 714 | 3,662 | 3,854 | 3,764 | 2,887 |
| Willow Pass Off | 913 | 858 | 775 | 738 | 149 | 145 | 136 | 145 | 764 | 714 | 639 | 593 |
| Mainline | 3,482 | 3,824 | 3,828 | 2,862 | 586 | 685 | 704 | 569 | 2,898 | 3,140 | 3,125 | 2,294 |
| Willow Pass On | 738 | 811 | 958 | 808 | 144 | 158 | 186 | 200 | 594 | 653 | 771 | 608 |
| Mainline | 4,220 | 4,635 | 4,786 | 3,670 | 730 | 843 | 890 | 769 | 3,492 | 3,793 | 3,896 | 2,902 |
| Burnett Off | 556 | 545 | 506 | 448 | 90 | 93 | 89 | 87 | 466 | 452 | 418 | 361 |
| Mainline | 3,664 | 4,090 | 4,280 | 3,222 | 640 | 750 | 801 | 682 | 3,026 | 3,341 | 3,478 | 2,541 |
| Burnett On | 469 | 490 | 524 | 434 | 84 | 87 | 94 | 78 | 385 | 403 | 430 | 356 |
| Mainline | 4,133 | 4,580 | 4,804 | 3,656 | 724 | 837 | 895 | 760 | 3,411 | 3,744 | 3,908 | 2,897 |
| Concord On | 284 | 317 | 362 | 279 | 96 | 85 | 74 | 76 | 188 | 232 | 288 | 203 |
| Mainline: NB I-680 north of Concord | 4,417 | 4,897 | 5,166 | 3,935 | 820 | 922 | 969 | 836 | 3,599 | 3,976 | 4,196 | 3,100 |

Note: Trucks are included in the SOV demand volume.

| ALTERNATIVE 6, 6A | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 6,322 | 6,228 | 6,328 | 5,943 | 740 | 903 | 877 | 765 | 5,582 | 5,325 | 5,451 | 5,178 |
| Bollinger Off-Ramp | 1,084 | 1,151 | 1,870 | 1,170 | 176 | 224 | 197 | 154 | 908 | 927 | 1,673 | 1,016 |
| Mainline | 5,238 | 5,077 | 4,458 | 4,773 | 564 | 679 | 680 | 611 | 4,674 | 4,398 | 3,778 | 4,162 |
| Bollinger Loop On | 306 | 253 | 214 | 255 | 61 | 60 | 56 | 63 | 245 | 193 | 158 | 192 |
| Mainline | 5,544 | 5,330 | 4,672 | 5,028 | 625 | 739 | 736 | 674 | 4,919 | 4,591 | 3,936 | 4,354 |
| Bollinger Diagonal On | 904 | 1,029 | 860 | 786 | 174 | 198 | 130 | 118 | 730 | 831 | 730 | 668 |
| Mainline | 6,448 | 6,359 | 5,532 | 5,814 | 799 | 937 | 866 | 792 | 5,649 | 5,422 | 4,666 | 5,022 |
| Crow Canyon Off-Ramp | 1,383 | 1,149 | 1,860 | 1,302 | 114 | 139 | 131 | 90 | 1,269 | 1,010 | 1,729 | 1,212 |
| Mainline | 5,065 | 5,210 | 3,672 | 4,512 | 685 | 798 | 735 | 702 | 4,380 | 4,412 | 2,937 | 3,810 |
| Crow Canyon Loop On | 678 | 798 | 713 | 604 | 139 | 182 | 180 | 155 | 539 | 616 | 533 | 449 |
| Mainline | 5,743 | 6,008 | 4,385 | 5,116 | 824 | 980 | 915 | 857 | 4,919 | 5,028 | 3,470 | 4,259 |
| Crow Canyon Diagonal On | 1,011 | 1,112 | 1,057 | 759 | 203 | 253 | 284 | 189 | 808 | 859 | 773 | 570 |
| Mainline | 6,754 | 7,120 | 5,442 | 5,875 | 1,027 | 1,233 | 1,199 | 1,046 | 5,727 | 5,887 | 4,243 | 4,829 |
| Sycamore Off-Ramp | 610 | 835 | 513 | 712 | 171 | 177 | 158 | 176 | 439 | 658 | 355 | 536 |
| Mainline | 6,144 | 6,285 | 4,929 | 5,163 | 856 | 1,056 | 1,041 | 870 | 5,288 | 5,229 | 3,888 | 4,293 |
| Sycamore On | 1,177 | 1,105 | 1,072 | 995 | 270 | 259 | 238 | 215 | 907 | 846 | 834 | 780 |
| Mainline | 7,321 | 7,390 | 6,001 | 6,158 | 1,126 | 1,315 | 1,279 | 1,085 | 6,195 | 6,075 | 4,722 | 5,073 |
| Diablo Valley Off-Ramp | 760 | 764 | 679 | 713 | 83 | 82 | 54 | 43 | 677 | 682 | 625 | 670 |
| Mainline | 6,561 | 6,626 | 5,322 | 5,445 | 1,043 | 1,233 | 1,225 | 1,042 | 5,518 | 5,393 | 4,097 | 4,403 |
| Diablo Valley Loop On | 336 | 238 | 264 | 223 | 67 | 57 | 69 | 54 | 269 | 181 | 195 | 169 |
| Mainline | 6,897 | 6,864 | 5,586 | 5,668 | 1,110 | 1,290 | 1,294 | 1,096 | 5,787 | 5,574 | 4,292 | 4,572 |
| Diablo Valley Diagonal On | 174 | 221 | 212 | 172 | 36 | 53 | 55 | 43 | 138 | 168 | 157 | 129 |
| Mainline | 7,071 | 7,085 | 5,798 | 5,840 | 1,146 | 1,343 | 1,349 | 1,139 | 5,925 | 5,742 | 4,449 | 4,701 |
| El Cerro Off-Ramp | 447 | 511 | 463 | 463 | 44 | 53 | 33 | 25 | 403 | 458 | 430 | 438 |
| Mainline | 6,624 | 6,574 | 5,335 | 5,377 | 1,102 | 1,290 | 1,316 | 1,114 | 5,522 | 5,284 | 4,019 | 4,263 |
| El Cerro On | 697 | 429 | 473 | 391 | 120 | 127 | 125 | 96 | 577 | 302 | 348 | 295 |
| Mainline | 7,321 | 7,003 | 5,808 | 5,768 | 1,222 | 1,417 | 1,441 | 1,210 | 6,099 | 5,586 | 4,367 | 4,558 |
| El Pintado On | 131 | 101 | 89 | 62 | 27 | 25 | 24 | 16 | 104 | 76 | 65 | 46 |
| Mainline | 7,452 | 7,104 | 5,897 | 5,830 | 1,249 | 1,442 | 1,465 | 1,226 | 6,203 | 5,662 | 4,432 | 4,604 |
| Stone Valley Diagonal Off | 189 | 201 | 193 | 151 | 26 | 35 | 36 | 27 | 163 | 166 | 157 | 124 |
| Mainline | 7,263 | 6,903 | 5,704 | 5,679 | 1,223 | 1,407 | 1,429 | 1,199 | 6,040 | 5,496 | 4,275 | 4,480 |
| Stone Valley Loop Off | 315 | 239 | 222 | 194 | 43 | 41 | 37 | 29 | 272 | 198 | 185 | 165 |
| Mainline | 6,948 | 6,664 | 5,482 | 5,485 | 1,180 | 1,366 | 1,392 | 1,170 | 5,768 | 5,298 | 4,090 | 4,315 |
| Stone Valley On | 612 | 557 | 516 | 458 | 91 | 96 | 92 | 74 | 521 | 461 | 424 | 384 |
| Mainline | 7,560 | 7,221 | 5,998 | 5,943 | 1,271 | 1,462 | 1,484 | 1,244 | 6,289 | 5,759 | 4,514 | 4,699 |


| ALTERNATIVE 6, 6A | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 256 | 214 | 247 | 234 | 36 | 36 | 42 | 39 | 220 | 178 | 205 | 195 |
| Mainline | 7,304 | 7,007 | 5,751 | 5,709 | 1,235 | 1,426 | 1,442 | 1,205 | 6,069 | 5,581 | 4,309 | 4,504 |
| Livorna On | 434 | 268 | 244 | 205 | 55 | 51 | 49 | 40 | 379 | 217 | 195 | 165 |
| Mainline | 7,738 | 7,275 | 5,995 | 5,914 | 1,290 | 1,477 | 1,491 | 1,245 | 6,448 | 5,798 | 4,504 | 4,669 |
| Rudgear Off | 580 | 501 | 505 | 489 | 66 | 88 | 104 | 85 | 514 | 413 | 401 | 404 |
| Mainline | 7,158 | 6,774 | 5,490 | 5,425 | 1,224 | 1,389 | 1,387 | 1,160 | 5,934 | 5,385 | 4,103 | 4,265 |
| Rudgear On | 457 | 498 | 542 | 452 | 69 | 86 | 100 | 76 | 388 | 412 | 442 | 376 |
| Mainline | 7,615 | 7,272 | 6,032 | 5,877 | 1,293 | 1,475 | 1,487 | 1,236 | 6,322 | 5,797 | 4,545 | 4,641 |
| S. Main Off | 483 | 358 | 299 | 316 | 65 | 64 | 70 | 65 | 418 | 294 | 229 | 251 |
| Mainline | 7,132 | 6,914 | 5,733 | 5,561 | 1,228 | 1,411 | 1,417 | 1,171 | 5,904 | 5,503 | 4,316 | 4,390 |
| Olympic Off | 587 | 966 | 852 | 791 | 63 | 134 | 154 | 97 | 524 | 832 | 698 | 694 |
| Mainline | 6,545 | 5,948 | 4,881 | 4,770 | 1,165 | 1,277 | 1,263 | 1,074 | 5,380 | 4,671 | 3,618 | 3,696 |
| SR 24 Off | 1,464 | 1,422 | 1,236 | 1,367 | 188 | 253 | 260 | 216 | 1,276 | 1,169 | 976 | 1,151 |
| Mainline | 5,081 | 4,526 | 3,645 | 3,403 | 977 | 1,024 | 1,003 | 858 | 4,104 | 3,502 | 2,642 | 2,545 |
| Olympic On | 1,355 | 1,135 | 1,518 | 1,075 | 188 | 235 | 254 | 170 | 1,167 | 900 | 1,264 | 905 |
| Mainline | 6,436 | 5,661 | 5,163 | 4,478 | 1,165 | 1,259 | 1,257 | 1,028 | 5,271 | 4,402 | 3,906 | 3,450 |
| Ygnacio Valley Off | 851 | 885 | 695 | 733 | 124 | 144 | 141 | 143 | 727 | 741 | 554 | 590 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| SR 24 On | 5,196 | 5,647 | 5,769 | 4,727 | 495 | 538 | 503 | 446 | 4,701 | 5,109 | 5,266 | 4,281 |
| Mainline | 10,781 | 10,423 | 10,237 | 8,472 | 1,536 | 1,653 | 1,619 | 1,331 | 9,245 | 8,770 | 8,618 | 7,141 |
| N. Main Off/Treat Off | 2,091 | 1,799 | 1,828 | 1,948 | 326 | 316 | 318 | 300 | 1,765 | 1,483 | 1,510 | 1,648 |
| Mainline | 8,690 | 8,624 | 8,409 | 6,524 | 1,210 | 1,337 | 1,301 | 1,031 | 7,480 | 7,287 | 7,108 | 5,493 |
| Lawrence On | 1,504 | 1,415 | 1,095 | 1,158 | 227 | 247 | 193 | 198 | 1,277 | 1,168 | 902 | 960 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Treat Off (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Truck Scales On (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Buskirk On | 989 | 1,331 | 1,362 | 973 | 193 | 219 | 265 | 228 | 796 | 1,112 | 1,097 | 745 |
| Mainline | 11,183 | 11,370 | 10,866 | 8,655 | 1,630 | 1,803 | 1,759 | 1,457 | 9,553 | 9,567 | 9,107 | 7,198 |
| Oak On | 463 | 545 | 722 | 580 | 91 | 107 | 142 | 123 | 372 | 438 | 580 | 457 |
| Mainline | 11,646 | 11,915 | 11,588 | 9,235 | 1,721 | 1,910 | 1,901 | 1,580 | 9,925 | 10,005 | 9,687 | 7,655 |
| Contra Costa Off | 702 | 662 | 820 | 671 | 102 | 105 | 129 | 107 | 600 | 557 | 691 | 564 |
| Mainline | 10,944 | 11,253 | 10,768 | 8,564 | 1,619 | 1,805 | 1,772 | 1,473 | 9,325 | 9,448 | 8,996 | 7,091 |
| Monument Off | 982 | 944 | 825 | 645 | 150 | 154 | 141 | 117 | 832 | 790 | 684 | 528 |


| ALTERNATIVE 6, 6A | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 9,962 | 10,309 | 9,943 | 7,919 | 1,469 | 1,651 | 1,631 | 1,356 | 8,493 | 8,658 | 8,312 | 6,563 |
| Monument On | 1,179 | 1,238 | 1,431 | 1,139 | 230 | 240 | 274 | 279 | 949 | 998 | 1,157 | 860 |
| Mainline | 11,141 | 11,547 | 11,374 | 9,058 | 1,699 | 1,891 | 1,905 | 1,635 | 9,442 | 9,656 | 9,469 | 7,423 |
| SR 242 Off | 5,582 | 5,625 | 5,553 | 4,496 | 835 | 919 | 920 | 793 | 4,747 | 4,706 | 4,633 | 3,703 |
| Mainline | 5,559 | 5,922 | 5,821 | 4,562 | 864 | 972 | 985 | 842 | 4,695 | 4,950 | 4,836 | 3,720 |
| Willow Pass Off | 1,141 | 1,073 | 969 | 922 | 186 | 181 | 170 | 181 | 955 | 892 | 799 | 741 |
| Mainline | 4,418 | 4,849 | 4,852 | 3,640 | 678 | 791 | 815 | 661 | 3,740 | 4,058 | 4,037 | 2,979 |
| Willow Pass On | 922 | 1,014 | 1,197 | 1,010 | 180 | 198 | 233 | 250 | 742 | 816 | 964 | 760 |
| Mainline | 5,340 | 5,863 | 6,049 | 4,650 | 858 | 989 | 1,048 | 911 | 4,482 | 4,874 | 5,001 | 3,739 |
| Burnett Off | 695 | 681 | 633 | 560 | 113 | 116 | 111 | 109 | 582 | 565 | 522 | 451 |
| Mainline | 4,645 | 5,182 | 5,416 | 4,090 | 745 | 873 | 937 | 802 | 3,900 | 4,309 | 4,479 | 3,288 |
| Burnett On | 586 | 613 | 655 | 542 | 105 | 109 | 117 | 97 | 481 | 504 | 538 | 445 |
| Mainline | 5,231 | 5,795 | 6,071 | 4,632 | 850 | 982 | 1,054 | 899 | 4,381 | 4,813 | 5,017 | 3,733 |
| Concord On | 355 | 396 | 452 | 349 | 120 | 106 | 92 | 95 | 235 | 290 | 360 | 254 |
| Mainline: NB I-680 north of Concord | 5,586 | 6,191 | 6,523 | 4,981 | 970 | 1,088 | 1,146 | 994 | 4,616 | 5,103 | 5,377 | 3,987 |

Note: Trucks are included in the SOV demand volume.

| ALTERNATIVE 7 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 6,322 | 6,228 | 6,328 | 5,943 | 740 | 903 | 877 | 765 | 5,582 | 5,325 | 5,451 | 5,178 |
| Bollinger Off-Ramp | 1,084 | 1,151 | 1,870 | 1,170 | 176 | 224 | 197 | 154 | 908 | 927 | 1,673 | 1,016 |
| Mainline | 5,238 | 5,077 | 4,458 | 4,773 | 564 | 679 | 680 | 611 | 4,674 | 4,398 | 3,778 | 4,162 |
| Bollinger Loop On | 306 | 253 | 214 | 255 | 61 | 60 | 56 | 63 | 245 | 193 | 158 | 192 |
| Mainline | 5,544 | 5,330 | 4,672 | 5,028 | 625 | 739 | 736 | 674 | 4,919 | 4,591 | 3,936 | 4,354 |
| Bollinger Diagonal On | 904 | 1,029 | 860 | 786 | 174 | 198 | 130 | 118 | 730 | 831 | 730 | 668 |
| Mainline | 6,448 | 6,359 | 5,532 | 5,814 | 799 | 937 | 866 | 792 | 5,649 | 5,422 | 4,666 | 5,022 |
| Crow Canyon Off-Ramp | 1,383 | 1,149 | 1,860 | 1,302 | 114 | 139 | 131 | 90 | 1,269 | 1,010 | 1,729 | 1,212 |
| Mainline | 5,065 | 5,210 | 3,672 | 4,512 | 685 | 798 | 735 | 702 | 4,380 | 4,412 | 2,937 | 3,810 |
| Crow Canyon Loop On | 678 | 798 | 713 | 604 | 139 | 182 | 180 | 155 | 539 | 616 | 533 | 449 |
| Mainline | 5,743 | 6,008 | 4,385 | 5,116 | 824 | 980 | 915 | 857 | 4,919 | 5,028 | 3,470 | 4,259 |
| Crow Canyon Diagonal On | 1,011 | 1,112 | 1,057 | 759 | 203 | 253 | 284 | 189 | 808 | 859 | 773 | 570 |
| Mainline | 6,754 | 7,120 | 5,442 | 5,875 | 1,027 | 1,233 | 1,199 | 1,046 | 5,727 | 5,887 | 4,243 | 4,829 |
| Sycamore Off-Ramp | 610 | 835 | 513 | 712 | 171 | 177 | 158 | 176 | 439 | 658 | 355 | 536 |
| Mainline | 6,144 | 6,285 | 4,929 | 5,163 | 856 | 1,056 | 1,041 | 870 | 5,288 | 5,229 | 3,888 | 4,293 |
| Sycamore On | 1,177 | 1,105 | 1,072 | 995 | 270 | 259 | 238 | 215 | 907 | 846 | 834 | 780 |
| Mainline | 7,321 | 7,390 | 6,001 | 6,158 | 1,126 | 1,315 | 1,279 | 1,085 | 6,195 | 6,075 | 4,722 | 5,073 |
| Diablo Valley Off-Ramp | 760 | 764 | 679 | 713 | 83 | 82 | 54 | 43 | 677 | 682 | 625 | 670 |
| Mainline | 6,561 | 6,626 | 5,322 | 5,445 | 1,043 | 1,233 | 1,225 | 1,042 | 5,518 | 5,393 | 4,097 | 4,403 |
| Diablo Valley Loop On | 336 | 238 | 264 | 223 | 67 | 57 | 69 | 54 | 269 | 181 | 195 | 169 |
| Mainline | 6,897 | 6,864 | 5,586 | 5,668 | 1,110 | 1,290 | 1,294 | 1,096 | 5,787 | 5,574 | 4,292 | 4,572 |
| Diablo Valley Diagonal On | 174 | 221 | 212 | 172 | 36 | 53 | 55 | 43 | 138 | 168 | 157 | 129 |
| Mainline | 7,071 | 7,085 | 5,798 | 5,840 | 1,146 | 1,343 | 1,349 | 1,139 | 5,925 | 5,742 | 4,449 | 4,701 |
| El Cerro Off-Ramp | 447 | 511 | 463 | 463 | 44 | 53 | 33 | 25 | 403 | 458 | 430 | 438 |
| Mainline | 6,624 | 6,574 | 5,335 | 5,377 | 1,102 | 1,290 | 1,316 | 1,114 | 5,522 | 5,284 | 4,019 | 4,263 |
| El Cerro On | 697 | 429 | 473 | 391 | 120 | 127 | 125 | 96 | 577 | 302 | 348 | 295 |
| Mainline | 7,321 | 7,003 | 5,808 | 5,768 | 1,222 | 1,417 | 1,441 | 1,210 | 6,099 | 5,586 | 4,367 | 4,558 |
| El Pintado On | 131 | 101 | 89 | 62 | 27 | 25 | 24 | 16 | 104 | 76 | 65 | 46 |
| Mainline | 7,452 | 7,104 | 5,897 | 5,830 | 1,249 | 1,442 | 1,465 | 1,226 | 6,203 | 5,662 | 4,432 | 4,604 |
| Stone Valley Diagonal Off | 189 | 201 | 193 | 151 | 26 | 35 | 36 | 27 | 163 | 166 | 157 | 124 |
| Mainline | 7,263 | 6,903 | 5,704 | 5,679 | 1,223 | 1,407 | 1,429 | 1,199 | 6,040 | 5,496 | 4,275 | 4,480 |
| Stone Valley Loop Off | 315 | 239 | 222 | 194 | 43 | 41 | 37 | 29 | 272 | 198 | 185 | 165 |
| Mainline | 6,948 | 6,664 | 5,482 | 5,485 | 1,180 | 1,366 | 1,392 | 1,170 | 5,768 | 5,298 | 4,090 | 4,315 |
| Stone Valley On | 612 | 557 | 516 | 458 | 91 | 96 | 92 | 74 | 521 | 461 | 424 | 384 |
| Mainline | 7,560 | 7,221 | 5,998 | 5,943 | 1,271 | 1,462 | 1,484 | 1,244 | 6,289 | 5,759 | 4,514 | 4,699 |


| ALTERNATIVE 7 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 256 | 214 | 247 | 234 | 36 | 36 | 42 | 39 | 220 | 178 | 205 | 195 |
| Mainline | 7,304 | 7,007 | 5,751 | 5,709 | 1,235 | 1,426 | 1,442 | 1,205 | 6,069 | 5,581 | 4,309 | 4,504 |
| Livorna On | 434 | 268 | 244 | 205 | 55 | 51 | 49 | 40 | 379 | 217 | 195 | 165 |
| Mainline | 7,738 | 7,275 | 5,995 | 5,914 | 1,290 | 1,477 | 1,491 | 1,245 | 6,448 | 5,798 | 4,504 | 4,669 |
| Rudgear Off | 580 | 501 | 505 | 489 | 66 | 88 | 104 | 85 | 514 | 413 | 401 | 404 |
| Mainline | 7,158 | 6,774 | 5,490 | 5,425 | 1,224 | 1,389 | 1,387 | 1,160 | 5,934 | 5,385 | 4,103 | 4,265 |
| Rudgear On | 457 | 498 | 542 | 452 | 69 | 86 | 100 | 76 | 388 | 412 | 442 | 376 |
| Mainline | 7,615 | 7,272 | 6,032 | 5,877 | 1,293 | 1,475 | 1,487 | 1,236 | 6,322 | 5,797 | 4,545 | 4,641 |
| S. Main Off | 483 | 358 | 299 | 316 | 65 | 64 | 70 | 65 | 418 | 294 | 229 | 251 |
| Mainline | 7,132 | 6,914 | 5,733 | 5,561 | 1,228 | 1,411 | 1,417 | 1,171 | 5,904 | 5,503 | 4,316 | 4,390 |
| Olympic Off | 587 | 966 | 852 | 791 | 63 | 134 | 154 | 97 | 524 | 832 | 698 | 694 |
| Mainline | 6,545 | 5,948 | 4,881 | 4,770 | 1,165 | 1,277 | 1,263 | 1,074 | 5,380 | 4,671 | 3,618 | 3,696 |
| SR 24 Off/Ygnacio Valley Off | 2,315 | 2,307 | 1,931 | 2,100 | 312 | 397 | 401 | 359 | 2,003 | 1,910 | 1,530 | 1,741 |
| Mainline | 4,230 | 3,641 | 2,950 | 2,670 | 853 | 880 | 862 | 715 | 3,377 | 2,761 | 2,088 | 1,955 |
| Olympic On | 1,355 | 1,135 | 1,518 | 1,075 | 188 | 235 | 254 | 170 | 1,167 | 900 | 1,264 | 905 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| Ygnacio Valley Off (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| SR 24 On | 5,196 | 5,647 | 5,769 | 4,727 | 495 | 538 | 503 | 446 | 4,701 | 5,109 | 5,266 | 4,281 |
| Mainline | 10,781 | 10,423 | 10,237 | 8,472 | 1,536 | 1,653 | 1,619 | 1,331 | 9,245 | 8,770 | 8,618 | 7,141 |
| N. Main Off/Treat Off | 2,091 | 1,799 | 1,828 | 1,948 | 326 | 316 | 318 | 300 | 1,765 | 1,483 | 1,510 | 1,648 |
| Mainline | 8,690 | 8,624 | 8,409 | 6,524 | 1,210 | 1,337 | 1,301 | 1,031 | 7,480 | 7,287 | 7,108 | 5,493 |
| Lawrence On | 1,504 | 1,415 | 1,095 | 1,158 | 227 | 247 | 193 | 198 | 1,277 | 1,168 | 902 | 960 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Treat Off (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Truck Scales On (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Buskirk On | 989 | 1,331 | 1,362 | 973 | 193 | 219 | 265 | 228 | 796 | 1,112 | 1,097 | 745 |
| Mainline | 11,183 | 11,370 | 10,866 | 8,655 | 1,630 | 1,803 | 1,759 | 1,457 | 9,553 | 9,567 | 9,107 | 7,198 |
| Oak On | 463 | 545 | 722 | 580 | 91 | 107 | 142 | 123 | 372 | 438 | 580 | 457 |
| Mainline | 11,646 | 11,915 | 11,588 | 9,235 | 1,721 | 1,910 | 1,901 | 1,580 | 9,925 | 10,005 | 9,687 | 7,655 |
| Contra Costa Off | 702 | 662 | 820 | 671 | 102 | 105 | 129 | 107 | 600 | 557 | 691 | 564 |
| Mainline | 10,944 | 11,253 | 10,768 | 8,564 | 1,619 | 1,805 | 1,772 | 1,473 | 9,325 | 9,448 | 8,996 | 7,091 |
| Monument Off | 982 | 944 | 825 | 645 | 150 | 154 | 141 | 117 | 832 | 790 | 684 | 528 |


| ALTERNATIVE 7 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 9,962 | 10,309 | 9,943 | 7,919 | 1,469 | 1,651 | 1,631 | 1,356 | 8,493 | 8,658 | 8,312 | 6,563 |
| Monument On | 1,179 | 1,238 | 1,431 | 1,139 | 230 | 240 | 274 | 279 | 949 | 998 | 1,157 | 860 |
| Mainline | 11,141 | 11,547 | 11,374 | 9,058 | 1,699 | 1,891 | 1,905 | 1,635 | 9,442 | 9,656 | 9,469 | 7,423 |
| SR 242 Off | 5,582 | 5,625 | 5,553 | 4,496 | 835 | 919 | 920 | 793 | 4,747 | 4,706 | 4,633 | 3,703 |
| Mainline | 5,559 | 5,922 | 5,821 | 4,562 | 864 | 972 | 985 | 842 | 4,695 | 4,950 | 4,836 | 3,720 |
| Willow Pass Off | 1,141 | 1,073 | 969 | 922 | 186 | 181 | 170 | 181 | 955 | 892 | 799 | 741 |
| Mainline | 4,418 | 4,849 | 4,852 | 3,640 | 678 | 791 | 815 | 661 | 3,740 | 4,058 | 4,037 | 2,979 |
| Willow Pass On | 922 | 1,014 | 1,197 | 1,010 | 180 | 198 | 233 | 250 | 742 | 816 | 964 | 760 |
| Mainline | 5,340 | 5,863 | 6,049 | 4,650 | 858 | 989 | 1,048 | 911 | 4,482 | 4,874 | 5,001 | 3,739 |
| Burnett Off | 695 | 681 | 633 | 560 | 113 | 116 | 111 | 109 | 582 | 565 | 522 | 451 |
| Mainline | 4,645 | 5,182 | 5,416 | 4,090 | 745 | 873 | 937 | 802 | 3,900 | 4,309 | 4,479 | 3,288 |
| Burnett On | 586 | 613 | 655 | 542 | 105 | 109 | 117 | 97 | 481 | 504 | 538 | 445 |
| Mainline | 5,231 | 5,795 | 6,071 | 4,632 | 850 | 982 | 1,054 | 899 | 4,381 | 4,813 | 5,017 | 3,733 |
| Concord On | 355 | 396 | 452 | 349 | 120 | 106 | 92 | 95 | 235 | 290 | 360 | 254 |
| Mainline: NB I-680 north of Concord | 5,586 | 6,191 | 6,523 | 4,981 | 970 | 1,088 | 1,146 | 994 | 4,616 | 5,103 | 5,377 | 3,987 |

Note: Trucks are included in the SOV demand volume.

| ALTERNATIVE 9 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline: NB I-680 south of Bollinger | 6,322 | 6,228 | 6,328 | 5,943 | 740 | 903 | 877 | 765 | 5,582 | 5,325 | 5,451 | 5,178 |
| Bollinger Off-Ramp | 1,084 | 1,151 | 1,870 | 1,170 | 176 | 224 | 197 | 154 | 908 | 927 | 1,673 | 1,016 |
| Mainline | 5,238 | 5,077 | 4,458 | 4,773 | 564 | 679 | 680 | 611 | 4,674 | 4,398 | 3,778 | 4,162 |
| Bollinger Loop On | 306 | 253 | 214 | 255 | 61 | 60 | 56 | 63 | 245 | 193 | 158 | 192 |
| Mainline | 5,544 | 5,330 | 4,672 | 5,028 | 625 | 739 | 736 | 674 | 4,919 | 4,591 | 3,936 | 4,354 |
| Bollinger Diagonal On | 904 | 1,029 | 860 | 786 | 174 | 198 | 130 | 118 | 730 | 831 | 730 | 668 |
| Mainline | 6,448 | 6,359 | 5,532 | 5,814 | 799 | 937 | 866 | 792 | 5,649 | 5,422 | 4,666 | 5,022 |
| Crow Canyon Off-Ramp | 1,383 | 1,149 | 1,860 | 1,302 | 114 | 139 | 131 | 90 | 1,269 | 1,010 | 1,729 | 1,212 |
| Mainline | 5,065 | 5,210 | 3,672 | 4,512 | 685 | 798 | 735 | 702 | 4,380 | 4,412 | 2,937 | 3,810 |
| Crow Canyon Loop On | 678 | 798 | 713 | 604 | 139 | 182 | 180 | 155 | 539 | 616 | 533 | 449 |
| Mainline | 5,743 | 6,008 | 4,385 | 5,116 | 824 | 980 | 915 | 857 | 4,919 | 5,028 | 3,470 | 4,259 |
| Crow Canyon Diagonal On | 1,011 | 1,112 | 1,057 | 759 | 203 | 253 | 284 | 189 | 808 | 859 | 773 | 570 |
| Mainline | 6,754 | 7,120 | 5,442 | 5,875 | 1,027 | 1,233 | 1,199 | 1,046 | 5,727 | 5,887 | 4,243 | 4,829 |
| Sycamore Off-Ramp | 610 | 835 | 513 | 712 | 171 | 177 | 158 | 176 | 439 | 658 | 355 | 536 |
| Mainline | 6,144 | 6,285 | 4,929 | 5,163 | 856 | 1,056 | 1,041 | 870 | 5,288 | 5,229 | 3,888 | 4,293 |
| Sycamore On | 1,177 | 1,105 | 1,072 | 995 | 270 | 259 | 238 | 215 | 907 | 846 | 834 | 780 |
| Mainline | 7,321 | 7,390 | 6,001 | 6,158 | 1,126 | 1,315 | 1,279 | 1,085 | 6,195 | 6,075 | 4,722 | 5,073 |
| Diablo Valley Off-Ramp | 760 | 764 | 679 | 713 | 83 | 82 | 54 | 43 | 677 | 682 | 625 | 670 |
| Mainline | 6,561 | 6,626 | 5,322 | 5,445 | 1,043 | 1,233 | 1,225 | 1,042 | 5,518 | 5,393 | 4,097 | 4,403 |
| Diablo Valley Loop On | 336 | 238 | 264 | 223 | 67 | 57 | 69 | 54 | 269 | 181 | 195 | 169 |
| Mainline | 6,897 | 6,864 | 5,586 | 5,668 | 1,110 | 1,290 | 1,294 | 1,096 | 5,787 | 5,574 | 4,292 | 4,572 |
| Diablo Valley Diagonal On | 174 | 221 | 212 | 172 | 36 | 53 | 55 | 43 | 138 | 168 | 157 | 129 |
| Mainline | 7,071 | 7,085 | 5,798 | 5,840 | 1,146 | 1,343 | 1,349 | 1,139 | 5,925 | 5,742 | 4,449 | 4,701 |
| El Cerro Off-Ramp | 447 | 511 | 463 | 463 | 44 | 53 | 33 | 25 | 403 | 458 | 430 | 438 |
| Mainline | 6,624 | 6,574 | 5,335 | 5,377 | 1,102 | 1,290 | 1,316 | 1,114 | 5,522 | 5,284 | 4,019 | 4,263 |
| El Cerro On | 697 | 429 | 473 | 391 | 120 | 127 | 125 | 96 | 577 | 302 | 348 | 295 |
| Mainline | 7,321 | 7,003 | 5,808 | 5,768 | 1,222 | 1,417 | 1,441 | 1,210 | 6,099 | 5,586 | 4,367 | 4,558 |
| El Pintado On | 131 | 101 | 89 | 62 | 27 | 25 | 24 | 16 | 104 | 76 | 65 | 46 |
| Mainline | 7,452 | 7,104 | 5,897 | 5,830 | 1,249 | 1,442 | 1,465 | 1,226 | 6,203 | 5,662 | 4,432 | 4,604 |
| Stone Valley Diagonal Off | 189 | 201 | 193 | 151 | 26 | 35 | 36 | 27 | 163 | 166 | 157 | 124 |
| Mainline | 7,263 | 6,903 | 5,704 | 5,679 | 1,223 | 1,407 | 1,429 | 1,199 | 6,040 | 5,496 | 4,275 | 4,480 |
| Stone Valley Loop Off | 315 | 239 | 222 | 194 | 43 | 41 | 37 | 29 | 272 | 198 | 185 | 165 |
| Mainline | 6,948 | 6,664 | 5,482 | 5,485 | 1,180 | 1,366 | 1,392 | 1,170 | 5,768 | 5,298 | 4,090 | 4,315 |
| Stone Valley On | 612 | 557 | 516 | 458 | 91 | 96 | 92 | 74 | 521 | 461 | 424 | 384 |
| Mainline | 7,560 | 7,221 | 5,998 | 5,943 | 1,271 | 1,462 | 1,484 | 1,244 | 6,289 | 5,759 | 4,514 | 4,699 |


| ALTERNATIVE 9 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Livorna Off | 256 | 214 | 247 | 234 | 36 | 36 | 42 | 39 | 220 | 178 | 205 | 195 |
| Mainline | 7,304 | 7,007 | 5,751 | 5,709 | 1,235 | 1,426 | 1,442 | 1,205 | 6,069 | 5,581 | 4,309 | 4,504 |
| Livorna On | 434 | 268 | 244 | 205 | 55 | 51 | 49 | 40 | 379 | 217 | 195 | 165 |
| Mainline | 7,738 | 7,275 | 5,995 | 5,914 | 1,290 | 1,477 | 1,491 | 1,245 | 6,448 | 5,798 | 4,504 | 4,669 |
| Rudgear Off | 580 | 501 | 505 | 489 | 66 | 88 | 104 | 85 | 514 | 413 | 401 | 404 |
| Mainline | 7,158 | 6,774 | 5,490 | 5,425 | 1,224 | 1,389 | 1,387 | 1,160 | 5,934 | 5,385 | 4,103 | 4,265 |
| Rudgear On | 457 | 498 | 542 | 452 | 69 | 86 | 100 | 76 | 388 | 412 | 442 | 376 |
| Mainline | 7,615 | 7,272 | 6,032 | 5,877 | 1,293 | 1,475 | 1,487 | 1,236 | 6,322 | 5,797 | 4,545 | 4,641 |
| S. Main Off | 483 | 358 | 299 | 316 | 65 | 64 | 70 | 65 | 418 | 294 | 229 | 251 |
| Mainline | 7,132 | 6,914 | 5,733 | 5,561 | 1,228 | 1,411 | 1,417 | 1,171 | 5,904 | 5,503 | 4,316 | 4,390 |
| Olympic Off | 587 | 966 | 852 | 791 | 63 | 134 | 154 | 97 | 524 | 832 | 698 | 694 |
| Mainline | 6,545 | 5,948 | 4,881 | 4,770 | 1,165 | 1,277 | 1,263 | 1,074 | 5,380 | 4,671 | 3,618 | 3,696 |
| SR 24 Off/Ygnacio Valley Off | 2,315 | 2,307 | 1,931 | 2,100 | 312 | 397 | 401 | 359 | 2,003 | 1,910 | 1,530 | 1,741 |
| Mainline | 4,230 | 3,641 | 2,950 | 2,670 | 853 | 880 | 862 | 715 | 3,377 | 2,761 | 2,088 | 1,955 |
| Olympic On | 1,355 | 1,135 | 1,518 | 1,075 | 188 | 235 | 254 | 170 | 1,167 | 900 | 1,264 | 905 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| Ygnacio Valley Off (Closed) | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| Mainline | 5,585 | 4,776 | 4,468 | 3,745 | 1,041 | 1,115 | 1,116 | 885 | 4,544 | 3,661 | 3,352 | 2,860 |
| SR 24 On | 5,196 | 5,647 | 5,769 | 4,727 | 495 | 538 | 503 | 446 | 4,701 | 5,109 | 5,266 | 4,281 |
| Mainline | 10,781 | 10,423 | 10,237 | 8,472 | 1,536 | 1,653 | 1,619 | 1,331 | 9,245 | 8,770 | 8,618 | 7,141 |
| N. Main Off | 370 | 343 | 340 | 333 | 57 | 60 | 60 | 51 | 313 | 283 | 280 | 282 |
| Mainline | 10,411 | 10,080 | 9,897 | 8,139 | 1,479 | 1,593 | 1,559 | 1,280 | 8,932 | 8,487 | 8,338 | 6,859 |
| Lawrence On | 1,504 | 1,415 | 1,095 | 1,158 | 227 | 247 | 193 | 198 | 1,277 | 1,168 | 902 | 960 |
| Mainline | 11,915 | 11,495 | 10,992 | 9,297 | 1,706 | 1,840 | 1,752 | 1,478 | 10,209 | 9,655 | 9,240 | 7,819 |
| Treat Off | 1,723 | 1,461 | 1,494 | 1,622 | 269 | 256 | 258 | 249 | 1,454 | 1,205 | 1,236 | 1,373 |
| Mainline | 10,192 | 10,034 | 9,498 | 7,675 | 1,437 | 1,584 | 1,494 | 1,229 | 8,755 | 8,450 | 8,004 | 6,446 |
| Truck Scales On | 2 | 5 | 6 | 7 | 0 | 0 | 0 | 0 | 2 | 5 | 6 | 7 |
| Mainline | 10,194 | 10,039 | 9,504 | 7,682 | 1,437 | 1,584 | 1,494 | 1,229 | 8,757 | 8,455 | 8,010 | 6,453 |
| Buskirk On | 989 | 1,331 | 1,362 | 973 | 193 | 219 | 265 | 228 | 796 | 1,112 | 1,097 | 745 |
| Mainline | 11,183 | 11,370 | 10,866 | 8,655 | 1,630 | 1,803 | 1,759 | 1,457 | 9,553 | 9,567 | 9,107 | 7,198 |
| Oak On | 463 | 545 | 722 | 580 | 91 | 107 | 142 | 123 | 372 | 438 | 580 | 457 |
| Mainline | 11,646 | 11,915 | 11,588 | 9,235 | 1,721 | 1,910 | 1,901 | 1,580 | 9,925 | 10,005 | 9,687 | 7,655 |
| Contra Costa Off | 702 | 662 | 820 | 671 | 102 | 105 | 129 | 107 | 600 | 557 | 691 | 564 |
| Mainline | 10,944 | 11,253 | 10,768 | 8,564 | 1,619 | 1,805 | 1,772 | 1,473 | 9,325 | 9,448 | 8,996 | 7,091 |
| Monument Off | 982 | 944 | 825 | 645 | 150 | 154 | 141 | 117 | 832 | 790 | 684 | 528 |


| ALTERNATIVE 9 | 2020 Total Demand Volumes |  |  |  | 2020 HOV Demand Volumes |  |  |  | 2020 SOV Demand Volumes |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Location | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM | 3-4 PM | 4-5 PM | 5-6 PM | 6-7 PM |
| Mainline | 9,962 | 10,309 | 9,943 | 7,919 | 1,469 | 1,651 | 1,631 | 1,356 | 8,493 | 8,658 | 8,312 | 6,563 |
| Monument On | 1,179 | 1,238 | 1,431 | 1,139 | 230 | 240 | 274 | 279 | 949 | 998 | 1,157 | 860 |
| Mainline | 11,141 | 11,547 | 11,374 | 9,058 | 1,699 | 1,891 | 1,905 | 1,635 | 9,442 | 9,656 | 9,469 | 7,423 |
| SR 242 Off | 5,582 | 5,625 | 5,553 | 4,496 | 835 | 919 | 920 | 793 | 4,747 | 4,706 | 4,633 | 3,703 |
| Mainline | 5,559 | 5,922 | 5,821 | 4,562 | 864 | 972 | 985 | 842 | 4,695 | 4,950 | 4,836 | 3,720 |
| Willow Pass Off | 1,141 | 1,073 | 969 | 922 | 186 | 181 | 170 | 181 | 955 | 892 | 799 | 741 |
| Mainline | 4,418 | 4,849 | 4,852 | 3,640 | 678 | 791 | 815 | 661 | 3,740 | 4,058 | 4,037 | 2,979 |
| Willow Pass On | 922 | 1,014 | 1,197 | 1,010 | 180 | 198 | 233 | 250 | 742 | 816 | 964 | 760 |
| Mainline | 5,340 | 5,863 | 6,049 | 4,650 | 858 | 989 | 1,048 | 911 | 4,482 | 4,874 | 5,001 | 3,739 |
| Burnett Off | 695 | 681 | 633 | 560 | 113 | 116 | 111 | 109 | 582 | 565 | 522 | 451 |
| Mainline | 4,645 | 5,182 | 5,416 | 4,090 | 745 | 873 | 937 | 802 | 3,900 | 4,309 | 4,479 | 3,288 |
| Burnett On | 586 | 613 | 655 | 542 | 105 | 109 | 117 | 97 | 481 | 504 | 538 | 445 |
| Mainline | 5,231 | 5,795 | 6,071 | 4,632 | 850 | 982 | 1,054 | 899 | 4,381 | 4,813 | 5,017 | 3,733 |
| Concord On | 355 | 396 | 452 | 349 | 120 | 106 | 92 | 95 | 235 | 290 | 360 | 254 |
| Mainline: NB I-680 north of Concord | 5,586 | 6,191 | 6,523 | 4,981 | 970 | 1,088 | 1,146 | 994 | 4,616 | 5,103 | 5,377 | 3,987 |

Note: Trucks are included in the SOV demand volume.

## YEAR 2020 SPEED CONTOUR MAPS FOR ALTERNATIVES

|  | 64 | 64 | 63 | 64 | 63 | 64 | 61 | 64 | 64 | 63 | 64 | 64 | 53 | 64 | 63 | 64 | 64 | 59 | 56 | 50 | 25 | 19 | 15 | 52 | 63 | 63 | 62 | 58 | 29 | 39 | 46 | 49 | 60 | 64 | 64 | 64 | 64 | 62 | 64 | 63 | 63 | 65 | 65 | 65 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3:30-4:00 | 65 | 65 | 64 | 64 | 64 | 65 | 62 | 64 | 64 | 64 | 64 | 22 | 16 | 19 | 17 | 13 | 12 | 13 | 24 | 19 | 16 | 17 | 15 | 48 | 28 | 16 | 11 |  | 9 | 12 | 25 | 45 | 60 | 63 | 63 | 64 | 64 | 62 | 64 | 62 | 62 | 65 | 64 | 65 | 64 | 62 |
|  | 65 | 65 | 64 | 64 | 64 | 65 | 63 | 64 | 64 | 64 | 30 | 8 | 8 |  |  |  | 8 | 9 | 15 | 11 | 10 | 11 |  | 10 |  | 8 |  |  |  | 12 | 28 | 46 | 61 | 64 | 63 | 64 | 64 | 61 | 64 | 61 | 63 | 65 | 64 | 65 | 64 | 63 |
| 4:30-5:00 | 64 | 64 | 63 | 64 | 60 | 65 | 62 | 61 | 38 | 25 | 11 | 4 | 4 | 4 |  |  |  | 5 | 9 | 10 | 9 | 11 | 10 | 12 | 12 | 10 |  |  | 10 | 13 | 28 | 45 | 61 | 64 | 63 | 64 | 63 | 58 | 64 | 61 | 62 | 64 | 64 | 64 | 64 | 62 |
|  | 65 | 65 | 64 | 65 | 54 | 13 | 9 |  |  | 5 | 5 | 6 | 7 |  |  |  |  | 7 | 11 | 10 | 9 | 11 | 9 | 10 | 10 | 10 |  | 9 | 11 | 16 | 35 | 45 | 60 | 64 | 62 | 64 | 47 | 48 | 64 | 61 | 63 | 65 | 64 | 64 | 63 | 61 |
|  | 64 | 64 | 44 | 31 | 17 | 7 | 7 | 8 |  | 5 | 5 | 5 | 5 | 5 | 6 | 5 | 6 | 8 | 15 | 16 | 14 | 15 | 13 | 14 | 13 | 11 | 9 | 10 | 11 | 15 | 32 | 43 | 59 | 61 | 44 | 26 | 21 | 45 | 64 | 60 | 63 | 64 | 63 | 64 | 63 |  |
|  | 66 | 66 | 39 | 33 | 14 | 8 | 8 | 13 | 6 | 8 | 6 | 9 | 9 | 10 | 10 | 8 | 10 | 11 | 19 | 18 | 16 | 17 | 14 | 13 | 12 | 11 |  | 10 | 11 | 14 | 30 | 45 | 60 | 58 | 51 | 37 | 27 | 44 | 64 | 60 | 62 | 64 | 64 | 64 | 63 | 61 |
|  | 64 | 64 | 61 | 63 | 61 | 48 | 37 | 20 | 10 | 12 | 10 | 11 | 12 | 11 | 11 | 9 | 10 | 10 | 20 | 17 | 15 | 15 | 13 | 12 | 12 | 11 | 9 | 12 | 12 | 17 | 36 | 46 | 60 | 64 | 64 | 64 | 64 | 60 | 65 | 63 | 63 | 65 | 65 | 65 | 64 | 63 |
| 2020 ALTERNATIVE 2 (RAMP METERING) GENERAL PURPOSE LANES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00-3:30 | 64 | 64 |  | 64 | 63 | 64 | 60 | 64 |  | 64 | 64 | 64 |  | 64 | 62 | 64 |  | 59 | 57 | 39 | 19 |  |  |  |  | 64 | 63 | 56 | 35 | 34 |  |  |  |  |  |  | 64 |  | 64 |  |  |  |  |  | 64 |  |
|  | 65 | 65 | 64 | 64 | 64 | 65 | 61 | 63 | 64 | 64 | 63 | 23 | 16 | 18 | 17 | 12 | 12 | 13 | 23 | 18 | 17 | 18 | 15 | 44 | 25 | 16 | 11 |  | 9 | 13 | 27 | 45 | 60 | 63 | 62 | 63 | 64 | 60 | 64 | 62 | 63 | 64 | 64 | 64 | 63 |  |
|  | 65 | 65 | 64 | 64 | 64 | 65 | 64 | 64 | 64 | 64 | 27 | 7 | 8 | 8 | 9 |  |  | 9 | 14 | 11 | 10 | 11 |  | 11 | 10 | 9 |  | 8 | 10 | 14 | 30 | 46 | 61 | 64 | 62 | 63 | 64 | 60 | 64 | 62 | 62 | 64 | 64 | 64 | 63 |  |
| 4:30-5:00 | 64 | 64 | 63 | 64 | 60 | 65 | 62 | 58 | 28 | 20 |  | 5 | 5 |  |  |  |  |  | 12 | 14 | 12 | 14 | 12 | 13 | 14 | 11 | 9 | 10 | 10 | 14 | 32 | 46 | 61 | 64 | 63 |  | 60 | 51 | 64 | 61 | 63 |  | 64 |  | 63 |  |
| 5:00-5 | 65 | 65 | 64 | 65 | 59 | 16 | 10 | 6 | 5 |  | 6 | 7 | 7 | 8 | 8 | 6 | 7 | 8 | 12 | 11 | 10 | 12 | 10 | 12 | 11 | 10 | 9 | 10 | 11 | 16 | 43 | 45 | 61 | 63 | 63 | 63 | 35 | 47 | 64 | 62 | 63 | 64 | 64 | 64 | 62 |  |
| 5:30-6:00 | 64 | 64 | 63 | 64 | 24 | 6 | 8 | 9 | 4 |  | 6 | 6 | 6 | 7 | 8 | 6 | 7 | 8 | 15 | 17 | 15 | 16 | 14 | 14 | 13 | 11 | 9 | 10 | 12 | 17 | 34 | 45 | 60 | 58 | 42 | 28 | 21 | 44 | 64 | 61 | 62 | 64 | 64 | 64 | 63 | 61 |
| 6:00-6:30 | 66 | 66 | 65 | 65 | 25 | 10 | 11 | 16 | 7 | 9 | 7 | 9 | 10 | 10 | 11 | 8 | 10 | 11 | 21 | 19 | 17 | 17 | 14 | 13 | 13 | 12 | 10 | 11 | 10 | 15 | 34 | 42 | 43 | 32 | 25 | 22 | 22 | 45 | 64 | 61 | 61 | 64 | 63 | 64 | 63 | 61 |
| 30-7 | 64 | 64 | 61 | 63 | 61 | 64 | 60 | 56 | 24 | 20 | 11 | 12 | 12 | 12 | 12 | 9 | 11 | 12 | 24 | 26 | 19 | 19 | 15 | 16 | 17 | 16 | 15 | 17 | 16 | 23 | 54 | 51 | 61 | 63 | 41 | 22 | 20 | 44 | 65 | 63 | 63 | 65 | 65 | 65 | 64 |  |




 6:00-6:30

| 3:00-3:30 | 65 | 65 | 65 | 65 | 64 | 65 | 63 | 65 | 65 | 64 | 65 | 65 | 60 | 65 | 65 | 65 | 65 | 64 | 62 | 64 | 64 | 64 | 65 | 62 | 61 | 63 | 62 | 62 | 65 | 60 | 47 | 64 | 64 | 63 | 61 | 62 | 62 | 59 | 61 | 55 | 60 | 64 | 65 | 64 | 64 | 63 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 3:30-4:00 | 65 | 65 | 65 | 65 | 64 | 65 | 64 | 65 | 65 | 65 | 65 | 64 | 52 | 64 | 64 | 64 | 59 | 22 | 47 | 62 | 64 | 50 | 24 | 55 | 61 | 64 | 63 | 63 | 65 | 48 | 38 | 64 | 65 | 63 | 58 | 62 | 62 | 59 | 55 | 50 | 60 | 64 | 64 | 64 | 64 | 63 |
| 4:00-4:30 | 65 | 66 | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 65 | 59 | 65 | 65 | 66 | 65 | 64 | 60 | 64 | 65 | 45 | 30 | 58 | 62 | 64 | 64 | 63 | 65 | 39 | 38 | 65 | 65 | 63 | 60 | 62 | 62 | 60 | 60 | 52 | 60 | 64 | 64 | 64 | 64 | 63 |
| 4:30-5:00 | 65 | 65 | 65 | 65 | 63 | 65 | 64 | 65 | 65 | 65 | 65 | 65 | 60 | 64 | 63 | 65 | 65 | 62 | 61 | 64 | 64 | 62 | 63 | 61 | 62 | 64 | 63 | 63 | 65 | 42 | 40 | 64 | 65 | 64 | 61 | 62 | 62 | 56 | 30 | 46 | 62 | 64 | 64 | 64 | 64 | 63 |
| 30 | 66 | 66 | 65 | 65 | 65 | 66 | 65 | 66 | 65 | 65 | 66 | 65 | 64 | 65 | 65 | 66 | 65 | 64 | 62 | 65 | 65 | 64 | 64 | 62 | 62 | 65 | 64 | 64 | 65 | 61 | 39 | 64 | 65 | 64 | 61 | 62 | 63 | 54 | 21 | 46 | 62 | 64 | 64 | 64 | 64 | 63 |
| 5:30-6:00 | 65 | 65 | 65 | 65 | 64 | 66 | 65 | 66 | 66 | 66 | 67 | 66 | 63 | 66 | 66 | 67 | 66 | 66 | 64 | 65 | 65 | 65 | 65 | 64 | 64 | 65 | 65 | 65 | 65 | 53 | 39 | 65 | 65 | 64 | 60 | 62 | 63 | 61 | 52 | 48 | 62 | 65 | 65 | 65 | 64 | 63 |
| 6:00-6:30 | 66 | 66 | 66 | 66 | 65 | 66 | 66 | 67 | 67 | 66 | 67 | 66 | 65 | 66 | 66 | 66 | 66 | 66 | 65 | 66 | 66 | 66 | 66 | 66 | 65 | 66 | 65 | 65 | 66 | 60 | 39 | 65 | 65 | 64 | 61 | 62 | 63 | 61 | 60 | 54 | 61 | 64 | 65 | 64 | 64 | 63 |
| 6:30-7:00 | 65 | 65 | 64 | 64 | 63 | 65 | 62 | 64 | 64 | 65 | 65 | 65 | 64 | 65 | 65 | 66 | 66 | 65 | 64 | 65 | 65 | 64 | 65 | 64 | 64 | 65 | 66 | 65 | 66 | 65 | 63 | 66 | 66 | 65 | 63 | 64 | 64 | 62 | 63 | 60 | 62 | 64 | 65 | 65 | 65 | 65 |
| 2020 ALTERNATIVE 5 (EXPRESS LANE EXTENSION AND GP LANE WIDENING) GENERAL PURPOSE LANES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 0-3:30 | 64 | 64 | 63 | 64 | 63 | 64 | 60 | 64 | 64 | 64 | 64 | 64 | 49 | 64 | 62 | 64 | 64 | 61 | 56 | 63 | 64 | 62 | 64 | 63 | 62 | 64 | 57 | 28 | 18 | 25 | 58 | 60 | 62 | 64 | 64 | 65 | 65 | 64 | 65 | 63 | 63 | 65 | 65 | 65 | 65 | 64 |
| 3:30-4:00 | 65 | 65 | 64 | 64 | 64 | 65 | 61 | 63 | 64 | 64 | 64 | 28 | 18 | 20 | 19 | 14 | 14 | 16 | 46 | 63 | 63 | 62 | 63 | 60 | 25 | 18 | 9 | 8 | 10 | 20 | 58 | 56 | 61 | 63 | 62 | 63 | 64 | 63 | 64 | 62 | 62 | 65 | 65 | 64 | 64 | 63 |
| 4:00-4:30 | 65 | 65 | 64 | 64 | 64 | 65 | 64 | 64 | 64 | 64 | 65 | 26 | 16 | 13 | 13 | 11 | 13 | 14 | 45 | 62 | 64 | 61 | 64 | 31 | 12 | 14 | 10 | 8 | 11 | 20 | 56 | 57 | 62 | 64 | 63 | 64 | 65 | 62 | 64 | 61 | 62 | 65 | 65 | 65 | 64 | 63 |
| 4:30 | 64 | 64 | 63 | 64 | 60 | 65 | 62 | 64 | 64 | 64 | 65 | 53 | 18 | 14 | 14 | 11 | 13 | 14 | 46 | 62 | 64 | 62 | 64 | 19 | 12 | 14 | 10 | 9 | 11 | 21 | 56 | 55 | 62 | 64 | 63 | 64 | 64 | 62 | 64 | 60 | 62 | 65 | 64 | 64 | 64 | 62 |
| 5:00 | 65 | 65 | 64 | 65 | 63 | 65 | 64 | 65 | 65 | 64 | 65 | 34 | 16 | 15 | 14 | 11 | 13 | 14 | 45 | 63 | 64 | 55 | 30 | 11 | 12 | 13 | 10 | 8 | 11 | 21 | 59 | 59 | 63 | 64 | 63 | 64 | 64 | 62 | 64 | 61 | 63 | 65 | 65 | 64 | 63 | 62 |
| 5: | 64 | 64 | 63 | 64 | 55 | 65 | 63 | 65 | 65 | 65 | 66 | 65 | 62 | 65 | 64 | 37 | 24 | 19 | 46 | 60 | 38 | 23 | 16 | 10 | 12 | 14 | 10 | 8 | 10 | 20 | 60 | 60 | 63 | 64 | 63 | 64 | 65 | 62 | 64 | 62 | 63 | 65 | 64 | 64 | 64 | 62 |
| 6: | 66 | 66 | 65 | 65 | 62 | 66 | 64 | 66 | 66 | 66 | 66 | 65 | 63 | 66 | 66 | 66 | 66 | 65 | 64 | 65 | 65 | 52 | 39 | 18 | 17 | 18 | 13 | 9 | 11 | 20 | 59 | 60 | 62 | 64 | 63 | 64 | 65 | 63 | 64 | 60 | 62 | 65 | 64 | 64 | 63 | 62 |
| 6:30-7:00 | 64 | 64 | 61 | 63 | 61 | 64 | 60 | 63 | 64 | 64 | 65 | 64 | 60 | 64 | 64 | 65 | 65 | 63 | 60 | 64 | 64 | 62 | 64 | 65 | 64 | 65 | 66 | 32 | 25 | 24 | 60 | 62 | 64 | 65 | 65 | 65 | 65 | 64 | 65 | 63 | 64 | 66 | 65 | 65 | 65 | 64 |
| 2020 ALTERNATIVE 6 (EXPRESS LANE EXTENSION AND GP LANE WIDENING PLUS C-D SYSTEM) GENERAL PURPOSE LANES |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| 3:00-3:30 | 64 | 64 | 63 | 64 | 63 | 64 | 60 | 64 | 64 | 64 | 64 | 64 | 52 | 64 | 62 | 64 | 64 | 61 | 56 | 63 | 64 | 63 | 64 | 64 | 62 | 63 | 63 | 52 | 22 | 55 | 63 | 63 | 64 | 62 | 59 | 62 | 63 | 61 | 62 | 60 | 61 | 63 | 63 | 63 | 63 | 62 |
| 3:30-4:00 | 65 | 65 | 64 | 64 | 64 | 65 | 61 | 63 | 64 | 64 | 64 | 33 | 19 | 19 | 18 | 14 | 13 | 15 | 46 | 62 | 63 | 59 | 62 | 62 | 63 | 63 | 64 | 40 | 24 | 41 | 64 | 63 | 63 | 53 | 49 | 60 | 62 | 57 | 62 | 55 | 59 | 63 | 63 | 63 | 62 | 61 |
| 4:00-4:30 | 65 | 65 | 64 | 64 | 64 | 65 | 64 | 64 | 64 | 64 | 65 | 25 | 14 | 13 | 14 | 11 | 13 | 14 | 46 | 62 | 64 | 61 | 63 | 63 | 63 | 64 | 64 | 65 | 63 | 61 | 64 | 64 | 64 | 63 | 57 | 61 | 62 | 60 | 62 | 60 | 59 | 63 | 63 | 63 | 63 | 61 |
| 4:30-5:00 | 64 | 64 | 63 | 64 | 60 | 65 | 62 | 64 | 64 | 64 | 65 | 41 | 16 | 13 | 14 | 11 | 13 | 14 | 45 | 63 | 64 | 61 | 63 | 63 | 63 | 64 | 65 | 65 | 64 | 61 | 64 | 63 | 64 | 63 | 57 | 61 | 62 | 59 | 62 | 59 | 61 | 63 | 63 | 63 | 62 | 61 |
| 5:00-5:30 | 65 | 65 | 64 | 65 | 63 | 65 | 64 | 65 | 65 | 64 | 65 | 25 | 14 | 14 | 14 | 11 | 13 | 14 | 46 | 62 | 64 | 59 | 63 | 64 | 63 | 64 | 65 | 65 | 63 | 61 | 63 | 63 | 64 | 63 | 58 | 61 | 63 | 58 | 63 | 61 | 61 | 63 | 63 | 63 | 62 | 60 |
| 5:30-6:00 | 64 | 64 | 63 | 64 | 55 | 65 | 63 | 65 | 65 | 65 | 66 | 65 | 62 | 65 | 54 | 30 | 23 | 18 | 45 | 63 | 64 | 61 | 64 | 64 | 63 | 64 | 64 | 65 | 63 | 60 | 63 | 63 | 64 | 60 | 55 | 60 | 62 | 57 | 63 | 59 | 61 | 64 | 63 | 62 | 62 | 59 |
| 6:00-6:30 | 66 | 66 | 65 | 65 | 62 | 66 | 64 | 66 | 66 | 66 | 66 | 65 | 63 | 66 | 66 | 66 | 66 | 65 | 64 | 65 | 65 | 64 | 65 | 66 | 65 | 65 | 66 | 66 | 65 | 63 | 64 | 64 | 64 | 64 | 59 | 62 | 63 | 61 | 63 | 58 | 61 | 63 | 63 | 63 | 62 | 60 |
| 6:30-7:00 | 64 | 64 | 61 | 63 | 61 | 64 | 60 | 63 | 64 | 64 | 65 | 64 | 62 | 64 | 64 | 65 | 65 | 63 | 61 | 64 | 64 | 63 | 64 | 65 | 64 | 64 | 65 | 66 | 65 | 64 | 65 | 65 | 65 | 65 | 62 | 63 | 64 | 63 | 64 | 63 | 63 | 64 | 64 | 65 | 64 | 63 |

$$
2020 \text { ALTERNATIVE 6A (GP LANE WIDENING PLUS C-D SYSTEM) GENERAL PURPOSE LANES }
$$



 | $4: 30-5: 00$ |
| :---: |
| $5 \cdot 00-5: 30$ |

## 





 2020 ALTERNATIVE 8 (CONTRA FLOW LANE PLUS EXPRESS LANE EXTENSION AND GP LANE WIDENING) GENERAL PURPOSE LANES








 |  | 64 | 16 | 15 | 14 | 11 | 13 | 14 | 45 | 63 | 64 | 60 | 63 | 63 |
| :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- | :--- |
| $5: 30-6: 00$ | 64 | 64 | 63 | 64 | 55 | 65 | 63 | 65 | 65 | 65 | 66 | 65 | 62 |
| 65 | 64 | 37 | 24 | 19 | 46 | 63 | 64 | 61 | 63 | 64 | 62 | 64 | 65 |
| $6: 00-6: 30$ | 66 | 66 | 65 | 65 | 62 | 66 | 64 | 66 | 66 | 66 | 66 | 65 | 63 |
| 66 | 66 | 66 | 66 | 65 | 64 | 65 | 65 | 64 | 65 | 66 | 64 | 65 | 66 |
| 66 | 66 | 62 | 56 | 55 | 62 | 63 | 62 | 63 | 64 | 62 | 64 |  |  |



## APPENDIX C - Cost Estimates

## SUMMARY OF ALTERNATIVE COSTS

| ALT | A |  | в |  | c |  | c2 | D |  | F1 |  | J |  | k |  | M1 |  | N |  | o |  | x |  | ESTIMATED <br> LOW | estimated <br> HIGH | $\begin{aligned} & \hline \text { USE } \\ & \text { Low } \end{aligned}$ | $\begin{gathered} \hline \text { USE } \\ \text { HIGH } \end{gathered}$ |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | Low | нІІн | Low | нІІн | Low | нІІн |  | Low | HIGH | Low | нIGH | Low | HIGH | Low | HIGH | Low | HIGH | Low | HIGH | Low | HIGH | Low | ніGH |  |  |  |  |
| 1 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | so | so | so | 50 |
| 2 | \$15,000,000 | \$20,00,000 |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |  | \$15,000,000 | \$20,000,000 | \$15,000,000 | \$20,000,000 |
| 3 | \$15,00,000 | \$20,000,000 | \$900,000 | \$1,100,000 | \$900,000 | \$1,100,000 | \$49,200,000 |  |  |  |  |  |  |  |  |  |  |  |  |  |  | \$18,00,000 | \$24,000,000 | \$84,000,000 | \$99,400,000 | 588,000,000 | \$96,000,000 |
| 4 | \$15,000,000 | \$20,000,000 | \$900,000 | \$1,100,000 | \$900,000 | \$1,100,000 | 549,200,000 |  |  | \$47,000,000 | \$58,000,000 |  |  |  |  |  |  |  |  |  |  | \$18,000,000 | \$24,000,000 | \$131,000,000 | \$153,400,000 | \$135,00,000 | \$155,000,000 |
| 5 | \$15,000,000 | \$20,000,000 | \$900,000 | \$1,100,000 |  |  |  |  |  |  |  | \$25,000,000 | \$30,000,000 | 56,500,000 | 58,00,000 | \$140,000,000 | \$160,000,000 |  |  |  |  | \$18,000,000 | \$24,000,000 | \$205,400,000 | \$223,100,000 | \$210,00,000 | \$250,000,000 |
| 6 | \$15,000,000 | \$20,000,000 | \$900,000 | \$1,100,000 |  |  |  |  |  | \$47,000,000 | \$58,000,000 | \$25,000,000 | \$30,000,000 | 56,50,000 | \$8,00,000 | \$140,000,000 | \$160,00,000 |  |  |  |  | \$18,000,000 | \$24,000,000 | \$252,400,000 | \$301,100,000 | \$250,00,000 | \$300,000,000 |
| 6A | \$15,00,000 | \$20,00,000 | \$900,000 | \$1,100,000 |  |  |  |  |  | \$47,000,000 | \$58,000,000 | \$25,000,000 | \$30,00,000 | 56,50,000 | \$8,00,000 |  |  |  |  |  |  | \$5,940,000 | \$7,92,000 | \$100,340,000 | \$125,020,000 | \$100,000,000 | \$125,000,000 |
| 7 | \$15,000,000 | \$20,00,000 | \$900,000 | \$1,100,000 |  |  |  |  |  | \$47,000,000 | \$58,000,000 | \$25,000,000 | \$30,00,000 |  |  | \$140,000,000 | \$160,00,000 |  |  | \$90,000,000 | \$100,000,000 | \$18,000,000 | \$24,000,000 | \$335,90,000 | \$393,100,000 | \$350,00,000 | 5400,000,000 |
| 8 | \$15,000,000 | \$20,00,000 | \$900,000 | \$1,100,000 |  |  |  | \$20,000,000 | \$25,00,000 |  |  | \$25,000,000 | \$30,000,000 |  |  | \$140,000,000 | \$160,000,000 |  |  |  |  | \$18,000,000 | \$24,000,000 | \$218,900,000 | \$260,10,000 | \$220,00,000 | \$260,000,000 |
| 9 | \$15,000,000 | \$20,000,000 | \$900,000 | \$1,100,000 |  |  |  |  |  |  |  | \$25,000,000 | \$30,000,000 | 56,50,000 | \$8,000,000 | \$40,000,000 | \$160,000,000 | \$500,000,000 | \$600,000,000 |  |  | \$18,000,000 | \$24,000,000 | \$705,400,000 | \$843,100,000 | \$700,00,000 | \$900,000,000 |

*C2 - Mode Shift Investment (see attached estimate)
Costs shown are construction estimates only and do no include $\mathrm{R} / \mathrm{W}$, Support Costs or $0 \& \mathrm{M}$





## CC 680 DAA GP Lane Conversion Alternatives (Alt 3 and 4)

## Cost for Express Buses

## Bus Seating Capacity - Assume 60 passengers per bus

- Mixed fleet of regular full size bus and double decker (50/50)
- Regular bus seating capacity: 57
- Double decker seating capacity: 80
- Average seating capacity: 68
- Assume < 90\% occupancy: 60 passengers per bus


## New Buses required: 42 buses

- 64 to 103 total for the peak period, let's be conservative and use the higher number 103 for cost estimates.
- Hourly distribution of buses required (3 to 7 PM): $28+27+26+22=103$
- Assume time required for a round trip for buses: 90 minutes
- New Buses required: $28+27 / 2=42$ buses
o 21 double decker
o 21 regular full size bus


## Capital Cost for New Buses: \$29.4M

- Double decker: $21 \times \$ 800 \mathrm{k}=\$ 16.8 \mathrm{M}$
- Regular full size: $21 \times \$ 600=\$ 12.6 \mathrm{M}$


## Annual O\&M for Buses: \$ 8.4M

- Unit cost for O\&M: \$175/hr
- 4 hours for AM and 4 hour for PM
- 42 buses
- 260 days/year
- $175 \times(4+4) \times 42 \times 260=\$ 15.3 \mathrm{M}$
- Assume $45 \%$ fare box recovery: $\$ 8.4 \mathrm{M}$


## Cost for Park-and-Ride

## Parking spaces: 3,300

- $50 \%$ of SOV reduction
- Potential expanded services from UBER/LYFT and other on-demand mobility may help reduce the need for park and ride, as those services would be provided from door-to-door


## Capital Cost for parking: \$19.8M

- $\quad \$ 6 \mathrm{k}$ per space, assumed surface lot, and does not include land cost


## Annual O\&M for parking: \$ 528k

- $\quad \$ 800$ per space, includes a full time security guard
- Assume 80\% of O\&M recovery from parking fees
- $\quad \$ 800 \times 3,300 \times 20 \%=\$ 528 \mathrm{k}$


## Ca I 680 Walnut Creek \#1603-0017B

March 4 ${ }^{\text {th }} 2016$

Carlton Haack
HDR
2379 Gateway Oaks Drive, Suite 200
Sacramento,
California 95833-4239

Via email carlton.haack@hdrinc.com

Project: Ca I 680 Walnut Creek
Dear Carl,

Attached please find our budget quotation for the Walnut Creek I 680 project. Prices will be adjusted and reflect increased costs that LTSSS experiences. For planning purposes please use an inflation factor of 3\% per year.

This proposal has capital costs for the system of $\$ 5,198,000$ for the barrier and one machine. Installation costs are likely another \$8-10 per foot or $\$ 80-110,000$ and could be accomplished in less than a week at night.

I have outlined a budget for operations costs at the end of the quote.

## Concrete Reactive Tension System (CRTS 18") Barrier (Purchase)

The total project as planned is currently $11,088 \mathrm{ft}(2.1$ miles) in one direction. Changes in Qty of plus or minus $10 \%$ will not affect the unit price.

All external steel shall be stainless steel or hot dipped galvanized in accordance with ASTM, A 123. The barriers do not include reflectors or striping.

18" CRTS system (TL-4) \$320.00 /Ft.

## Variable Length Barrier - VLB

VLB units will be substituted one for one for CRTS barrier pieces and will be charged at the same unit price as concrete CRTS units. Lindsay will provide the correct number of VLB's based on the final deployment drawings.

## Boston Class Barrier Transfer Machine (Purchase)

1 EA. Machine per Specification MS 110610 Rev 7 \$1,650,000.00 EA
Training - provided as part of the sales price
Lindsay will provide the following training prior to installation and for the first year of operation.

- Onsite installation oversight and training during initial barrier replacement.
- Full applications engineering support
- Live roadway training with operators for 1-2 weeks as necessary
- Maintenance training for mechanics
- During the remaining warranty period, our representative will visit the job periodically, but at least quarterly and provide a written report for management outlining system conditions and concerns if any.

The cost for this training is included with the purchase of the system.

## Taxes:

This quotation is exclusive of federal taxes, which the parties agree are the responsibility of the buyer unless an exemption letter is provided.

## Delivery:

All barriers and VLB's shall be manufactured or made available at casting yard within 100 miles of the project site. Buyer takes legal ownership and full risk of loss at the casting site after inspection and acceptance.

Said Products shall then be considered delivered to the Buyer on the basis of Ex-Works Seller's premises (ICC Incoterms 2010) upon the earlier of: (i) Buyer's inspection and written acceptance of the identified Products, or (ii) the date occurring 30 days after Seller's notice of inspection is sent and the Buyer has failed to perform an inspection and send written notice of acceptance or rejection within such time frame.

Delivery timelines:
Barrier and VLB's: All barriers will take up to 6 months to deliver Machine: Would take about 9 months to deliver

## Operations:

As Byron and I stated in our meeting LTSSS would be willing to operate the system for Caltrans or whichever entity is responsible for procurement and operations. Very preliminary estimates for the full cost including labor and parts would be between $\$ 350,000$ and $\$ 425,000$ per year. The size of the proposed system is similar to the one in Washington DC and public bids are consistent with our calculations. There are many variables that need to be clarified but that estimate should be reasonable for this early stage.

Cordially,
Lindsay Transportation Solutions Sales \& Service LLC


Chris Sanders

Senior Vice president

Jeff Shewmaker - LTS
Byron West -LTS

## APPENDIX D - DAA TAC Meeting Minutes

## DRAFT MEETING MINUTES

Contra Costa Northbound I-680 Design Alternative Assessment Kick-Off Meeting
October 26, 2015
2:00-4:00pm
CCTA Offices - 2999 Oak Road, Suite 100, Walnut Creek, CA
Action Items: see underlined italic text

1. Welcome \& Introduction
a. Self-Introduction
b. Roles \& Responsibilities
2. Project Background
a. Project Background and Recent Studies
i. Discussion of future projects on the 680 corridor
3. I-680 North Express Lanes Conversion - Currently in PAED \& Design
a. Southbound conversion of existing HOV lane. Construction will include widening to close the gap in the HOV lane from Treat Boulevard to Rudgear Road.
b. Leo Scott noted that this project no longer includes the conversion of the existing NB HOV lane from SR 242 to the Benicia Bridge
4. I-680 South Express Lane Conversion - Under Construction
a. Conversion of existing HOV lanes from Alcosta Road to Livorna Road.
5. I-680 Northbound Planned HOV/Express Lane - PSR (Caltrans 2007)
a. Extension of the NB HOV lane from N. Main overcrossing to SR 242
b. Leo Scott stated that this project could not be assumed as an existing condition of this study because he is concerned with the geometrics included in the PSR.
i. HDR to review PSR to identify any red flag geometric issues.
b. Purpose of this study:
i. Desired end product for MTC/CCTA is to include the alternatives from this study in a PAED effort for the northbound HOV gap closure.
ii. Also want to include in the Plan Bay Area 2040.

## 3. Traffic Conditions

a. Available Data and Traffic Models
i. Eddie Barrios discussed the existing traffic through the corridor, including discussion of increasing traffic pattern within the last two years.

1. Bottlenecks caused by combination of traffic demand over capacity, weaving, and geometry (grade at Livorna Rd.).
ii. Two bottlenecks identified
2. Livorna Rd.
a. Eddie stated that the bottleneck is exacerbated by the grade approaching Livorna.
b. Steve Waymire indicated that it was also an issue with weaving that influences the bottleneck.
c. Eddie stated that recent data shows the bottleneck at Livorna Rd. has shortened in the last two years extending to Crow Canyon Rd. instead of Bollinger Canyon Rd as previously. It is believed this is due to the recently constructed auxiliary lanes in that area.
3. North Main St./Treat Blvd.
a. This is the controlling bottleneck. Recent data showed that the queue from this bottleneck spills back to overlap with the Livorna bottleneck causing queuing to Crow Canyon.
4. Data suggests that traffic around Monument Blvd. is close to capacity.
iii. Walnut Creek has seen a 6\% growth in traffic since 2005

## b. Study Approach

i. Traffic analysis evaluation should be done at a corridor level to assessment system effects.
ii. Rob stated that from previous projects, the greatest benefit to traffic operations will be achieved if the HOV/Express Lane can be carried through all bottlenecks
iii. Steve indicated that he wants a fresh look and 'outside the box' solutions, i.e. interchange closures, moving structure columns, tunnels, etc.
iv. Ashley Nguyen asked what year the traffic analysis would analyze.

1. Eddie stated that they had planned to analyze 2040 to serve an eventual PSR/PDS.
2. The group thought that traffic would be so congested by 2040 that it would be difficult to compare any improvements between the alternatives.
3. Group decided that the more appropriate approach would be to analyze 2020.

## 4. Alternative Development Brainstorm

a. Alternatives identified in RFP
i. Conversion of General Purpose lane into an Express Lane:

1. Kevin stated that this this alternative needs to be assessed per MTC's policy.
2. Eddie stated that this would likely be feasible for short segments or in combination with other strategies to reduce congestion on the general purpose lanes.
ii. Contra-Flow:
3. Brian Stewart explained that the northbound express lane would travel on the southbound roadway (in the southbound express lane) during the PM peak. The contra-flow lane would extend through the 24 interchange.
a. Rob stated that the southbound express lane does show benefits in the PM peak period. Eddie stated that the analysis would need to include improvements to the southbound roadway to offset any negative impacts to PM traffic.
4. The barrier for northbound operation could be either moveable or fixed.
a. Group indicated that the fixed barrier would create an issue during the AM peak when the southbound express lane is in operation as continuous through the 24 interchange.
b. Susan and Leo indicated concern for storing a moveable barrier in the left shoulder at the pinch points at the BART and SR24 columns with less than 1 ' shoulders.
5. Group had a brief discussion of ingress and egress points.
6. The group had a positive reaction and wanted to pursue the concept further.
iii. Adaptive Ramp Metering:
7. Eddie stated that it had not yet been determined what type of analysis would be conducted on this alternative.
8. Steve indicated concern over "adaptive" ramp metering and its affects on the local roadway network.
9. Jerry Fahy indicated that they have similar concerns over the current ramp metering on SR 4.
iv. Collector Distributor Road: Sheena Patel stated the concept would provide a two lane off ramp for N. Main/Treat prior to the N. Main OC, one lane would peel off to the N . Main loop, the other would run parallel to the highway as a C-D road. Lawrence on ramp would tie into the C-D road, Treat off would exit, and the C-D road would re-enter 680 just beyond the existing truck scales.
10. Identified as a way of helping to relieve the Main/Treat bottleneck due to weaving.
11. Mike Kerns indicated that given the ramp volumes (on and off) a one or two lane collector distributor could be overly congested.
12. Steve expressed that the city would be willing to work with the project on resulting impacts to Corps yard.
v. HOV Direct Connector from Lawrence on ramp:
13. Lawrence Way is a heavily congested on ramp in the PM providing access from downtown Walnut Creek.
14. Eddie questioned whether the ramp had enough HOV volume to justify the cost of the structure.
a. Susan reminded the group that it would be an express lane allowing tolled vehicles to access as well which would reduce congestion on Lawrence Way.
15. Brian brought up concerns about how the touchdown would interact with a contra-flow lane or even an extension of the HOV lane south of N . Main.
16. Group thought that the concept should be reviewed at a high level to determine whether or not it could be eliminated.
vi. Removal of Truck Scale: Removal of truck scale at Treat Blvd. is thought to be beneficial if coupled with other alternatives due to the additional width it would provide for widening and/or collector distributor options.
17. Susan Miller mentioned that in previous meetings between CCTA and CHP, CHP expressed that they need the truck scales at Treat Blvd. to remain open. Using weigh-in-motion (WIM) technology as a possible replacement is a potential solution to reduce the overall footprint.
vii. Widening: Sheena mentioned that there is potential to widen between Livorna on-ramp to S. Main off-ramp in order to extend the limits of the existing Express Lane. It would require a tall retaining wall.
18. This extension would help address the bottleneck at Livorna but could push more traffic into the N . Main bottleneck and reducing the overall system effectiveness.
19. The group thought the alternative could also be used in combination with other alternatives like the contra-flow lane.

## b. Other Alternatives:

i. Kevin Chen presented an alternative concept with braided ramps at Lawrence on ramp and Treat off ramp.

1. Lawrence on ramp would go under Treat off so that the grade changes could be minimized.
2. Group agreed that this alternative was worth moving forward.
ii. Susan suggested a tunnel option to connect HOV lanes through the SR 24 interchange. Group discussed that the most likely entrance to the tunnel would be just after Mt. Diablo Blvd. and the exit would be north of the Trinity OC support columns.
3. HDR to review concept.
iii. Relocating/redesigning columns (SR 24, BART, Trinity Ave) to allow for additional width for various alternatives. Steve asked whether we were willing to assume this as a constraint not worth pursuing.
4. Group decided that the structure constraints at the SR24 interchange would need to be evaluated at a very high level and mentioned with a brief discussion indicating why they were not pursued.
iv. Susan mentioned shoulder running as an alternative, either in the median or outside shoulder.
5. Caltrans has been reluctant to allow it on the outside shoulder because of interference with on ramps. In this corridor, the outside shoulder is the only option since there is little to no inside shoulder.
6. Group agreed to assess the shoulder running lane at a high level.
7. Opportunities, Constraints, Fatal Flaws
8. Action Items \& Next Steps
9. Project Schedule
10. Future TAC Meetings

Contra Costa Northbound I-680 Design Alternative Assessment Technical Advisory Committee (TAC)

October 26, 2015, 2:00 PM
CCTA Offices | 2999 Oak Road, Suite 100, Walnut Creek, CA
SIGN-IN SHEET


## DRAFT MEETING MINUTES

Contra Costa Northbound I-680 Design Alternative Assessment Meeting \#2
November 17, 2015
3:15-5:00 pm
CCTA Offices - 2999 Oak Road, Suite 100, Walnut Creek, CA
Action Items: see underlined italic text

## 1. Introductions

2. Recap of Project Objectives
a. Kevin provided a summary of the project objectives
i. Find innovative ways to create a northbound express lane
ii. Alleviate congestion on northbound I-680
iii. Express lane on its own may not be sufficient to alleviate congestion and the team should consider other improvements to improve traffic operations. Other improvements currently under consideration include such things as C-D road and braided ramps system and auxiliary lanes.
b. Goal of this meeting is review the alternative concepts that have been discussed to date and offer an opportunity for the team to comment.
c. After the concepts are further along an evaluation criteria matrix will be developed and will be used to weigh the pros/cons of the alternatives.

## 3. Review Draft Alternatives Concepts

a. Sheena presented the alternatives currently under consideration
i. Year 2020 Baseline Conditions: This represents the expected condition by year 2020. It includes a continuous southbound I-680 express lane with two locations providing restrictive access. In the northbound direction an express lane will be provided south of Livorna. The current northbound HOV lane north of SR 242 is proposed to remain an HOV lane.
ii. Alternative 1 - Contra Flow Lane: This alternative would convert a GP lane from the Livorna on-ramp to Rudgear on-ramp to an express lane. To help offset the potential loss of capacity from the conversion the alternative would also widen the mainline on the outside from the Livorna on-ramp to the Rudgear on-ramp to provide a new GP lane. Near the northbound Rudgear on-ramp the northbound express lane would transition to the southbound I-680 express lane and operate as a contraflow lane roughly between the Rudgear on-ramp and N. Main Street Off-Ramp. The southbound I-680 express lane would be inoperable between the N. Main Street and Rudgear interchange during the operation of the contra flow lane. The existing HOV lane currently provided north of SR 242 would be extended to the N. Main Street off-ramp and converted to an express lane; thereby, providing the opportunity for a continuous northbound express lane from county line to county line. The team discussed the potential issues associated with how southbound traffic would transition in and out of the southbound express lane during the operation of the contra flow lane and how this could be potentially be designed. Another issue that was raised was the loss of capacity in the southbound direction as a result of taking a lane away and the potential to substantially worsen southbound traffic operations. The team requested that a high level capacity analysis be performed to determine the potential impacts to
southbound traffic operations. Fehr \& Peers with assistance from MTC to determine the potential impacts to southbound I-680.
iii. Alternative 2 - Tunnel Concept: This alternative is similar to the contra flow concept except it would construct a tunnel underneath the I-680/SR 24 interchange instead of using the I-680 southbound roadbed. Initial review of the tunnel concept indicates that grades may need to be as high as $15 \%$ to keep the tunnel as compact as possible. To provide a tunnel with grades more consistent with HDM guidance the tunnel would require tunneling over 60 feet below ground conditions due to existing structures. HDR indicated that as a side note the cost of the tunnel was comparable to reconstructing the I-680/SR 24 interchange. As such HDR began to look at a concept that involved reconstruction assuming the team was in support. The team indicated that they were interested in hearing HDR's thoughts on reconstructing the l-680/SR 24 interchange. A decision was made to discuss this concept after the team went through the alternatives currently under consideration. Based on the information provided by HDR on the tunnel concept and team feedback it was determined that this concept should not be explored further but should be documented in the final report with a discussion about the concept and decision of why it was decided not to explore further. HDR to provide write up in the Design Alternative Assessment Memorandum.
iv. Alternative 3 - Conversion of General Purpose Lane: This alternative would convert a GP lane from the Livorna on-ramp to the Olympic off-ramp to an express lane. To help offset the potential loss of capacity from the conversion the alternative would also widen the mainline on the outside from the Livorna onramp to the Rudgear on-ramp to provide a new GP lane. The existing HOV lane currently provided north of SR 242 would be extended to the N. Main Street offramp and converted to an express lane. With this alternative a gap in the express lane system would still remain between Olympic and Main Street. The team discussed that this is currently the most straightforward alternative and perhaps easiest to implement; however, the major drawback is that a gap in the northbound express lane system would remain, although the gap would be shorter.
v. Alternative 3 a - General Purpose Conversion with a Collector-Distributor (C-D) Road System: This alternative is the same as Alternative 3 except it would also provide a C-D road system to service the N. Main Street off-ramp, Lawrence Way on-ramp and Treat Boulevard off-ramp. A section of the C-D road would require three lanes to accommodate the high traffic demand volumes. The C-D road system is intended to address the existing bottleneck between the Lawrence Way on-ramp and Treat Boulevard off-ramp by moving weaving traffic between Lawrence on and Treat off from the mainline to the C-D system. The goal is to reduce congestion at this location such that northbound express lane users can easily weave over from the center lane approaching SR 24 to the left hand side. This alternative would require the relocation of the truck scales and have potential impacts to the Corps yard.
vi. Alternative 3b - General Purpose Conversion with Braided Ramps: This alternative is the same as Alternative 3a except that instead of provided a C-D road system to address weaving traffic the Lawrence on-ramp and Treat off-ramp would be braided.
vii. Alternative 3c - General Purpose Conversion with Direct Lawrence Way Connector: This alternative is the same as Alternative 3 on the south side of SR 24. Unlike Alternative 3, the existing HOV lane north of SR 242 would only be extended to about the Treat Boulevard off-ramp to allow for a direct HOV/express lane connector from Lawrence Way to the northbound express lane. Two key items were discussed regarding this alternative: 1) the direct connector would effectively prohibit the ability for the express lane to be extended further south in the future; and 2 ) the start of the express lane (i.e. around the Treat Boulevard off-ramp) would not be very effective in addressing the existing bottleneck between the Lawrence Way on-ramp and Treat Boulevard off-ramp. Based on the discussion it was determined that this concept should not be explored further but should be documented in the final report with a discussion about the concept and decision of why it was decided not to explore further. HDR to provide write up in the Design Alternative Assessment Memorandum.
viii. "Ross New Concept" - Sheena indicated that she did her best to interpret the schematic provided by Ross and that she would reach out to him to confirm her understanding was correct. The concept is similar to the contraflow lane concept except that this alternative would not use the southbound express lane but instead use the southbound median shoulder. Sheena indicated that although it seems that today there is an opportunity to use the shoulder unfortunately there would not be much a shoulder left after the Southbound HOV Gap Closure project is implemented as that project is using the existing shoulder as part of its design. HDR to confirm with Ross that their understanding of his concept is accurate.
ix. I-680/SR 24 Interchange Reconstruction Alternative - This was a new alternative brought up by HDR after their review of the existing constraint locations and very preliminary cost ranges for some of the alternatives under consideration. The general idea of the alternative would be to reconfigure the interchange such that SR 24 comes in from right hand side as opposed to the left hand side. This can potentially provide increased opportunities for a continuous express lane. HDR sketched out conceptually in front of the group how this would work and what the constraints were. The team indicated that they would like HDR to develop this concept further to potentially include as one of the alternatives for evaluation. As part of developing this concept further the team also asked HDR to look into the feasibility of providing an additional lane from the SR 24 off ramp to the Olympic Boulevard on-ramp. HDR to develop this alternative further and look into the feasibility of providing an additional lane from the SR 24 off ramp to the Olympic Boulevard on-ramp. HDR would also look into the constructability of this concept in terms of staging.

## 4. Next Steps

a. Next meeting to occur the second/third week of December.

Contra Costa Northbound I-680 Design Alternative Assessment Technical Advisory Committee (TAC)

November 17, 2015, 3:15 PM
CCTA Offices | 2999 Oak Road, Suite 100, Walnut Creek, CA
SIGN-IN SHEET


## DRAFT MEETING MINUTES

Contra Costa Northbound I-680 Design Alternative Assessment Meeting \#3
December 14, 2015
3:00-5:00 pm
CCTA Offices - 2999 Oak Road, Suite 100, Walnut Creek, CA
Action Items: see underlined italic text

## 1. Introductions and Recap

a. Kevin indicated that the minutes for the last meeting had been sent to team members and if there were any comments or feedback to send it to him.
b. Kevin indicated that the primary focus of today's meeting was to review the project alternatives that have been discussed to date including the concept developed by Ross. Present the proposed evaluation criteria and matrix. Finally, provide an update on the traffic analysis.

## 2. Evaluation Criteria/Matrix

a. Carl and Brian went down the list of the alternatives in the evaluation criteria/matrix and provided the group a brief description of the various concepts still under consideration. (Note: detailed descriptions of the concepts and schematics of the alternatives were provided as part of the minutes from the last meeting).
b. Brian gave a brief summary of HDR's meeting with Ross to discuss his concept. The concept entails using the southbound roadbed (primarily southbound shoulder) to provide a northbound express lane. Currently, there appears to be sufficient shoulder width to potentially consider this concept; however, the southbound express lanes gap closure project (to be completed by year 2020) will utilize the majority of the SB shoulder. After discussion it was concluded that this concept would be dropped from further consideration as part of this study but would be documented in the final report.
c. Brian provided additional information regarding the new concept of reconfiguring the I680/SR 24 interchange such that SR 24 eastbound would join I-680 northbound on the right side instead of the left side, permitting a continuous express lane on the left (\#1 lane) for NB I-680. HDR concluded that this concept was feasible but would likely have a high construction cost. The team decided that this concept warranted further consideration as part of this study. For this alternative, traffic analysis would be conducted as a first step. Then, depending on the level of potential traffic operational benefits, the team would decide whether to perform additional design and cost estimation work.
d. Ashley informed the group that it's MTC's policy to evaluate a general purpose lane to express lane conversion scenario. This alternative would include a lane conversion for both north and south of the SR 24 interchange, to the extent that it's feasible.
e. HDR guided the team through some of the constraints associated with extending the northern express lane to the south past the Main Street O/C. In particular, the retaining wall adjacent to the Marriott Hotel presents the major design/construction challenge. CCTA asked HDR to break their investigation of extending the express lane into two sets of design assumptions. The first set of assumptions could include design exceptions that
they feel could be potentially acceptable to Caltrans and use the existing roadway bed and right-of-way, while the second set assumptions could include modifications to the retaining wall or reduced lane widths. HDR to investigate further the constraints associated with extending the northern express lane.
f. The team spent some time discussing the importance of pulling the express lane further south past the North Main Street O/C which is the location where the Caltrans PSR assumed the HOV lane would start. Fehr \& Peers informed the team that the operational benefit of the express lane through the bottleneck between the Lawrence On and Treat Off could only be achieved if northbound vehicles had sufficient distance to transition from the general purpose lane to the express lane prior to the Lawrence on-ramp gore point. Based on previous analysis performed by Fehr \& Peers for CCTA, it appears that the HOV lane should start at about 1,500 feet south of the North Main Street O/C. The further south it starts the greater the operational benefit of the express lane.
g. Brian walked through the opportunities and constraints on the southern portion of the corridor in terms of widening for express lanes (between Livorna Road on-ramp and SR 24 on-ramp). As part of the discussion, he introduced the idea of reconfiguration the Ygnacio off-ramp, to be combined with the SR 24 off-ramp with a new connector to Ygnacio.

## 3. Traffic Analysis Update

a. Kevin and Eddie provided an update on the traffic analysis. The VISSIM model has been updated to reflect year 2015 conditions. Also, year 2020 No Build forecasts had been developed by assuming a $1 \%$ per year growth rate.
b. Kevin informed the group that this study should consider mode shift as a result of the build alternatives providing an improved HOV lane system. The group spent some time discussing the specifics and how mode shift would be considered in the traffic analysis and documentation. Eddie raised the question of whether or not this topic could be deferred to PA/ED. Ashley indicated that MTC was very interested in beginning the conversation now in this study as she was concerned that if it was deferred to PA/ED that a "business as usual" approach in PA/ED would not properly address mode shift.
c. The group also discussed about the potential for additional mode shift that could be achieved with further investments to enhance express bus services, park and ride lots, etc. These strategies would help reduce the overall vehicle trip demands on the corridor which serving the same number of persons more efficiently.
d. Due to time constraints it was determined to continue the discussion on mode shift assumptions for project alternatives at the next meeting.

## 4. Next Meeting

a. The next meeting is being planned for the $3^{\text {rd }}$ or $4^{\text {th }}$ week in January 2016. Kevin will send out a survey to the group to confirm a date that works for everyone.

## DRAFT MEETING MINUTES

## Contra Costa Northbound I-680 Design Alternative Assessment Meeting \#3 January 25, 2015 2:00 PM <br> CCTA Offices - 2999 Oak Road, Suite 100, Walnut Creek, CA <br> Action Items: see underlined italic text

## 1. Introductions and Recap

Kevin provided a recap of the project activities to date, and indicated that the minutes of the last meeting had been sent to team members. He also asked that any comments or feedback be sent to him.

## 2. Traffic Analysis Update

a. Eddie provided an update of the traffic analysis effort. The 2020 No Build analysis, which indicates that traffic congestion will worsen by 2020 when compared to existing conditions: freeway queue length will be longer and travel times increase.
b. The group discussed mode shift assumptions for Alternatives 3 and 4 and agreed to assume that a mode shift to more carpoolers and transit riders would occur under these two scenarios.
3. Geometric Concepts, Design Updates and Traffic Analysis Scenarios Matrix,
a. Carol and Brian provided an update of the design analysis efforts.
b. A handout was distributed to the group that provided a detailed list of design elements discussed to date, and a matrix of traffic analysis scenarios which combined various elements.
c. The group discussed and agreed on 9 scenarios to be carried forward for traffic analyses.
d. The group agreed that detailed design of ramp meters will not be included as part of this DAA effort.
e. For the C-D road design element, Brian will try to tighten the radius at off- and on-ramp to avoid encroaching the on the corp yard.
f. For alternative 8 (contra flow lane), Brian will evaluate potential design speeds other than those for the existing mainline.

## 4. Next Meeting

a. The next meeting is being planned for mid-March, 2016.

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## DRAFT MEETING MINUTES

Contra Costa Northbound I-680 Design Alternative Assessment Meeting
March 17, 2016
9:00 am - 12:00 pm
CCTA Offices - 2999 Oak Road, Suite 100, Walnut Creek, CA
Action Items: see underlined italic text

## 1. Introductions and Recap

Kevin provided a project overview and shared the agenda. He indicated that the primary goal of the meeting was to share the technical analysis results.

## 2. Draft Alternative Evaluation Results

a. Carl provided several handouts that presented the proposed alternatives. Each of the graphics for the alternatives included the geometric improvement concepts that it includes and a cost range.
b. Alternative 2 - Eddie discussed some of the benefits of Alternative 2 (Ramp Metering) and Brian shared some information regarding the geometry and assumptions used to develop capital construction cost range estimates. Team feedback was that the ramp meter construction costs may be on the low side and the team agreed to increase them. HDR to adjust ramp meter construction costs to \$15M to \$20M range (Geometric Improvement Concept $A$ ). The reasons for the likely higher costs were to potentially provide HOV bypass lane per ramp metering design standards and ROW acquisitions.
c. Alternative 3 - Eddie provided some information regarding the mode shift assumptions under this alternative. One of the major assumptions was the overall reduction of vehicle demand by $20 \%$. Toshi provided additional insight into the assumptions. In particular, that one of the objectives of this alternative was to find the necessary vehicle demand reduction that would result in this alternative providing a similar benefit to other alternatives. With an understanding of how much of a vehicle demand reduction was necessary they can begin to find multimodal strategies to achieve the reduction. Eddie indicated that Alternative 3 showed a substantial benefit over the baseline alternative Alternative 1. Given the benefit of Alternative 3 is largely due to the $20 \%$ reduction in vehicle demand the team requested that this information be clear in the documentation. Steve noted that the group should be prepared to respond to the questions: what will the freeway conditions be like without the mode shift in this alternative.

Fehr \& Peers to add notes to report that Alternative 3 needs to be implemented in coordination with other transit improvements and add notes where the mode shift was applied. HDR to add note to alternative exhibits indicating a $20 \%$ reduction to vehicle demand that reflects mode shift.

In the Draft DAA report, discussions will be added regarding the combined effort between this study, and CCTA's recently completed "I-680 Transit Investment/Congestion Relief Options Study" into an overall plan for encouraging a mode shift.

## FehrłPEERS

Brian provided some additional information regarding the geometry for Alternative 3 regarding the general purpose lane drop to HOV lane start at the Main Street interchange. The team discussed the need to clarify the current assumptions and future design considerations. Brian indicated that the current MUTCD supports a general purpose lane feeding an Express Lane directly without tapers (i.e. without a GP lane drop before HOV lane start). Leo indicated that Caltrans' initial preference is likely to see a lane drop before the HOV lane add. Eddie added that the traffic analysis was done with a lane drop before the HOV lane start. For the purposes of moving forward is was decided that this document should keep the GP lane drop before the HOV lane start but identify the need to consider and evaluate the benefits of having a GP lane feeding directly into an Express Lane. Ross mentioned that in the next phase, the project could consider adding a lane on the SR 24 connector prior to touch down at 680.

HDR to add modify Alternative 3 exhibit to show general purpose lane drop before HOV lane start. Fehr \& Peers to acknowledge in report the need to consider a GP lane feeding directly into the HOV lane in future studies and the likely benefits of this design. Fehr \& Peers to also clarify in the MOE Table that the results shown in "()" are compared to Alternative 1 (No Build).
d. Alternative 4 - Eddie presented the analysis results and assumptions for this alternative. Similar to Alternative 3 one of the major assumptions was the overall reduction of vehicle demand by $20 \%$. Eddie indicated that Alternative 4 showed the most benefit over the baseline alternative. A new bottleneck developed under this alternative on northbound I680 just after SR 242. This is because under current conditions there are three general purpose lanes at this location and under Alternative 4 there would be 1 express lane and two general purpose lanes resulting in an overall lower capacity. A suggestion to eliminate this bottleneck was to take one of the mandatory SR 242 exit lanes and make it an optional exit so that 3 GP lanes were provided to SR 242 and 3 GP lanes were provided to northbound I-680. It was determined that Alternative 4 would remain as is but that future studies could consider additional improvements.

Ross indicated that since this alternative provided the most benefit it would be worthwhile to approximate and document how many buses would be needed to realize a $20 \%$ vehicle demand reduction. Fehr \& Peers and MTC to estimate this value, and potential costs associated with it.

The team discussed that the C-D system assumed under this alternative was a big factor in the achieving the benefit. Brian indicated that the truck scales would need to be modified to accommodate the C-D system. The team asked HDR to further clarify the cost implications of modifying the trucks scales. HDR to do the following: 1) Include weigh in motion in cost range; 2) add costs for mitigation of removing the truck scales; 3) add costs of about \$5M to \$10M to City of WC (impact to yard).

Brian provided additional clarification regarding the proposed HOV to Express lane conversion north of SR 242 . He indicated that these costs were not currently included under any of the alternatives. He indicated that this would be corrected. HDR to the following: 1) Create a new Geometric Improvement Concept " $X$ " to denote the express lane conversion and include it under Alternatives 3 through 9 ; 2) include cost of about $\$ 1.5$ to $\$ 2 \mathrm{M}$ per lane mile for 12 miles; 3) document the assumptions of the cost estimate such as no backhaul, no support costs, etc.
e. Alternative 5 - Eddie presented the analysis results and assumptions for this alternative. This alternative does not assume any mode shift. Eddie indicated that Alternative 5 showed a substantial benefit over the baseline alternative.

Brian indicated that the cost was based on "narrow" solution and that extending the northern HOV lane to the south would require a design exception due to sight distance and the design speed would likely need to be around 45 mph . Brian asked the team if the cost should include the cost to fix the curve at SR 242 and avoid the sight distance issue. The team said yes. HDR to adjust costs for Express Lane extension from SR 242 (M1) to include higher costs from PSR (SR 242 Structure Replacement to fix design exception). Cost about an extra \$100M.

Given the projected high cost of structure replacement Susan asked HDR to look into a cost saving option. HDR to look into the feasibility of SR 242 exit lane option to help correct curve design exception.
f. Alternative 6 - Eddie presented the analysis results and assumptions for this alternative. This alternative does not assume any mode shift and is very similar to Alternative 5 except that in addition to the Express Lane Extension (M1) it also includes the C-D roadway system. The Express Lane Extension in combination with the C-D roadway system would eliminate almost all of the congestion north of SR 24.

Given the potential difficulty and high cost involved in implementing the Express Lane Extension south of SR-242, a new alternative (Alternative 6A) was discussed as a potential phasing strategy for Alternative 6. This alternative would be identical to Alternative 6 but it would not include the Express Lane Extension (M1). The results from Alternative 6A could potentially be used to determine the need for the inclusion of the Express Lane Extension in Alternative 6.
Fehr \& Peers to evaluate new Alternative 6A.
g. Alternative 7 - Eddie presented the analysis results and assumptions for this alternative. This alternative does not assume any mode shift. The traffic analysis results of Alternative 7 are nearly identical to Alternative 6. Brian pointed out that the cost of Alternative 7 was substantially higher than Alternative 6.
h. Alternative 8 - Eddie highlighted that Alternative 8, the contra flow lane alternative, was the first of two alternatives that provided a continuous express lane. The traffic analysis results of Alternative 8 are nearly identical to Alternative 6 and 7. Ross asked what the potential traffic issues might be for the southbound direction. Mike responded that based on his analysis there would be no near-term issues as the southbound bottleneck that develops at Livorna would continue to be the controlling bottleneck. Potential impacts on the southbound operations be unlikely until after Year 2030. However, in the future there potential issues might be expected as traffic demand continues to grow, but that queues from the Livorna bottleneck would extend through them before they had a chance to develop and overall there would not be any additional impact to southbound traffic. Brian pointed out that the current cost range does not include yearly O\&M costs. The team asked HDR to include O\&M costs to provide a more complete picture of costs and a better apples to apples cost comparison with the other alternatives. $\underline{H D R}$ to add year

## FehrłPeers

O\&M costs to Alternative 8. HDR will also need design speeds for the contra flow lane (likely at 50 mph ).
i. Alternative 9 - Eddie highlighted that Alternative 9, the reconfiguration of the I-680/SR 24 interchange, was the second alternative to provide a continuous express lane. The traffic analysis results of Alternative 9 are nearly identical to Alternatives 6, 7, and 8. Brian pointed out that the estimate cost for this alternative was significantly higher than the other alternatives.
j. The team discussed that strictly from a cost to benefit ratio that Alternative 7 and 9 should not be carried further in future studies. HDR to document this recommendation in the Design Alternatives Assessment (DAA) Report.
k. Toshi indicated that while only Alternative 3 and 4 explicitly consider mode shift that the team should look for strategies and resources to encourage mode shift for all alternatives. The team agreed. HDR to provide guidance on how the concepts presented in the DAA report can work together with transit improvements along the corridor to better frame the next steps of these efforts.

Contra Costa Northbound I-680 Design Alternative Assessment Technical Advisory Committee (TAC)

March 17, 2016, 9:00 AM
CCTA Offices | 2999 Oak Road, Suite 100, Walnut Creek, CA
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# Contra Costa Northbound I-680 Design Alternative Assessment 

Technical Advisory Committee (TAC) Meeting<br>May 11, 2016, 2:00 PM<br>CCTA Offices<br>2999 Oak Road, Suite 100, Walnut Creek, CA<br>Meeting Summary

## 1. Introductions

2. Power Point Presentation
a. Project Background and Purpose

- Kevin thanked the team members for completing the Draft DAA and for the close collaboration.
- Kevin indicated that the success of the project was in part due to leveraging available data and traffic models.
- Kevin indicated that this project really included out of the box ideas and concepts in addition to traditional capacity increasing alternatives.
- Kevin identified the project purpose to address the 7.5 mile gap in the northbound I-680 managed lane to reduce congestion and increase person \& vehicle throughput.


## b. Traffic Analysis Results - Alternative 6A

- Eddie presented Alternative 6A which is a new alternative and was based as a potential first phase to Alternative 6. It is similar to Alternative 6 except it does not include the southern extension of the northern managed lane.
- Based on the traffic analysis a bottleneck would develop between Monument off-ramp and Monument on-ramp at the mainline lane drop location. The queue from this bottleneck would extend upstream through the El Pintado bottleneck.
- Based on vehicle and person hours of delay Alternative 6A would operate worse than Alternative 6 but substantially better than Alternatives 1 and 2.


## c. Benefit Cost Analysis

- Brian provided a summary of the benefit cost evaluation that was used to rank the alternatives based on performance and cost. The ranking (from best to worst)
- Alternative 2
- Alternative 3
- Alternative 4
- Alternative 6A
- Alternative 8
- Alternative 5
- Alternative 6
- Alternative 7
- Eddie and Kevin added that this benefit cost analysis was not the typical analysis that is commonly done by Caltrans as part of their analysis but rather a more simplified version to get a sense of which alternatives could provide the biggest bang for the buck.


## d. Recommended Alternatives for Further Consideration

- Brian presented the team's current alternatives that are recommended to be studied further:
- Alternative 4
- Alternative 6
- Alternative 8
- Alternative 2
- Brian indicated that other alternatives could be initial phases of these alternatives. For example, Alternative 3 could be an initial phase of Alternative 4. Alternative 5 and 6A could be initial phases of Alternative 6
- Of the alternatives studied, only Alternative 7 and Alternative 9 were not recommended for further study. This is due to the challenges they will present in design and getting Caltrans concurrence. Furthermore, these two alternatives ranked last in the benefit to cost evaluation.
- Susan asked about the Caltrans factor. In particular, which alternatives are likely to give Caltrans more concern than others. The team discussed the unique elements of each of the Alternatives that would definitely require close coordination with Caltrans. Ultimately, Alternative 6 was viewed as the most traditional alternative that Caltrans is familiar with and would likely cause the least concern. Alternative 8 was identified as likely causing Caltrans the most concern due to its very unique design features and potential safety concerns. Since Alternative 8 is the only recommended alternative that fully closes the gap, the team determined that it warranted further study in the next project phase where a more detailed evaluation would occur.


## e. Project Next Steps

- Carl indicated that a potential next step could be a PSR/PDS to help keep the project going. Based on his experience, the DAA includes the majority of the elements required in the PSR/PDS and the only new element that would be needed would be a PEAR (Preliminary Environmental Analysis Report)
- The team acknowledged that further discussion was needed with Caltrans to better understand the next steps in project development including PSR/PDS and PA/ED.


## f. Questions?

## 3. Action Items \& Next Steps

- The Draft DAA is ready for team review. We are looking for comments within two weeks (by May 25).
- Final DAA will be provided in June.

Contra Costa Northbound I-680 Design Alternative Assessment Technical Advisory Committee (TAC)

May 11, 2016, 9:00 AM
CCTA Offices | 2999 Oak Road, Suite 100, Walnut Creek, CA
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## APPENDIX E - List of Design Exceptions

## SUMMARY OF MAJOR DESIGN EXCEPTIONS

| ALT | DESIGN CONCEPT | DESIGN EXCEPTION* |
| :---: | :---: | :---: |
| 1 | No Build | NA |
| 2 | A | Ramp Metering Policy Exception - For HOV By-Pass Lanes |
| 3 | $A+B+C+X$ | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer |
| 4 | $A+B+C+F 1+X$ | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer \& at the N. Main St Overcrossing (C-D Road Off-Ramp) |
| 5 | A+B+J+K+M1+X | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer \& along managed lane extension( $N$. Main to SR 242) |
| 6 | $\mathrm{A}+\mathrm{B}+\mathrm{F} 1+\mathrm{J}+\mathrm{K}+\mathrm{M} 1+\mathrm{X}$ | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer, at the N. Main St Overcrossing (C-D Road Off-Ramp) \& along managed lane extension(N. Main to SR 242) |
| 6 A | $A+B+F 1+J+K+X$ | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer \& at the N. Main St Overcrossing (C-D Road Off-Ramp) |
| 7 | $\mathrm{A}+\mathrm{B}+\mathrm{F} 1+\mathrm{J}+\mathrm{M} 1+\mathrm{O}+\mathrm{X}$ | Ramp Metering Policy Exception, Left Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer, at the N. Main St Overcrossing (C-D Road Off-Ramp) \& along managed lane extension(N. Main to SR 242) |
| 8 | $A+B+D+J+M 1+X$ | Ramp Metering Policy Exception, Left Shoulder Width, Right Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer, along the Contra Flow Lane (SR 24 to N. Main) \& along managed lane extension(N. Main to SR 242) |
| 9 | $A+B+J+K+M 1+N+X$ | Ramp Metering Policy Exception, Left Shoulder Width, Right Shoulder Width, Lane Width - For Ramp Metering, Express Lane signs and buffer, along managed lane extension(N. Main to SR 242) \& at the I-680/SR24 Interchange columns |

* A complete list of Caltrans Design Exceptions should be identified in the next phase of design


[^0]:    R/W: R/W may be required for the addition of HOV by-pass lanes at certain ramp locations. Constructability: The proposed work is straight forward and would

    ## Considerations

    require some temporary closures of the on-ramps. That work can be done at night with detours. Safety: There should be little concern with safety for this concept as long as the design meets current standards.[^1]:    ${ }^{1}$ This calculation includes only the congested miles and does not include the number of lanes provided segment by segment.

[^2]:    ${ }^{2}$ Note that while a comparison is made for VMT between Alternatives 3 and 4 with Alternative 1 it is not truly a direct comparison as Alternatives 3 and 4 have substantially lower demand volumes than Alternative 1 .

[^3]:    * Costs shown are construction estimates only and do not include support costs

